

*Reinforced-concrete structures*

# Span-to-depth ratio assessment



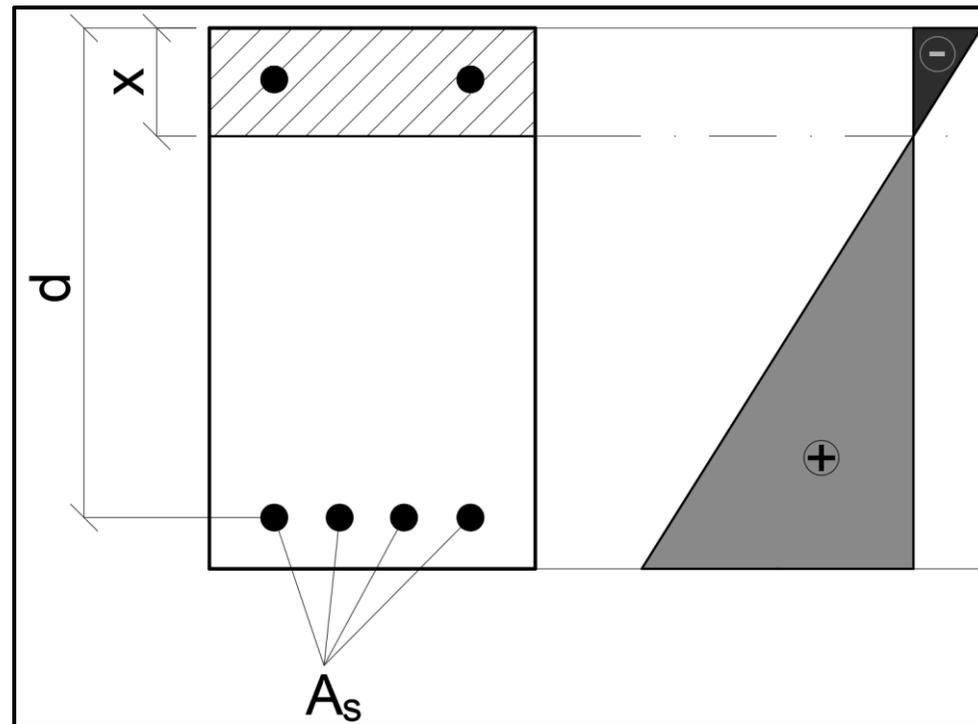
# Span-to-depth assessment

The effective depth is one of the **basic cross-section parameters**.

This parameter is **used in various equations**, and its **exact determination is therefore an important step** in the static calculation.

# Effective depth

The effective height ( $d$ ) is the distance **from the most compressed fiber of the concrete to the centroid of the tensile reinforcement.**



# Serviceability Limit States

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When assessing structures, in addition to the *Ultimate Limit States*<sup>\*</sup>, we must also consider the *Serviceability Limit States* – i.e., **check whether the structure is capable of fulfilling its functions during normal operation.**

# Serviceability Limit States

In practice, **we usually check three Serviceability Limit States:**

- **stress** limitation,
- **crack** control,
- **deflection** control.

In **this presentation**, we will focus only on the **deflection control**\*.

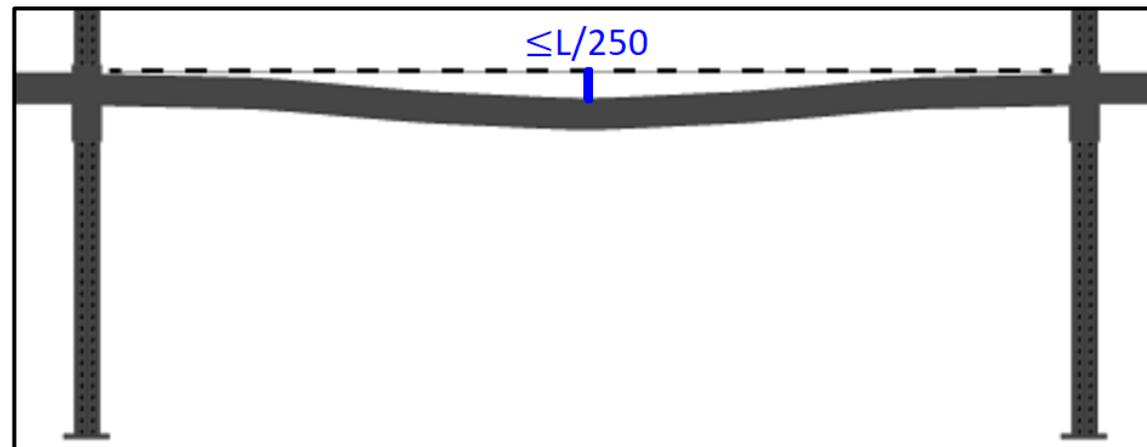
\* Other serviceability limit states are also very important, but their assessment is more complicated and will be described in future presentations.

# Deflection control

# Deflection control

The EC2 standard states that the **deformation** of the member **must not adversely affect** its **functionality** and **appearance**.

Specifically, the standard states that the **appearance and functionality may be compromised** if the calculated **deflection** (under quasi-permanent load) **exceeds 1/250 of the span**.



# Deflection control

Therefore, we just **need to calculate the deflection**

$$w = ?$$

and **compare it with the limit value**

$$w_{lim} = \frac{L}{250}.$$

However, it is **very difficult to calculate the deflection.**

Fortunately, the **standard provides a simplified approach** to the deflection control – a ***Span-to-depth ratio assessment.***

# Span-to-depth ratio assessment

# Span-to-depth ratio assessment

The standard states that if the Span-to-depth ratio of a beam ( $\lambda$ ) does not exceed its limiting value ( $\lambda_d$ ), i.e.:

$$\lambda \leq \lambda_d,$$

**it can be assumed that the deflections will not exceed the limit values\*.**

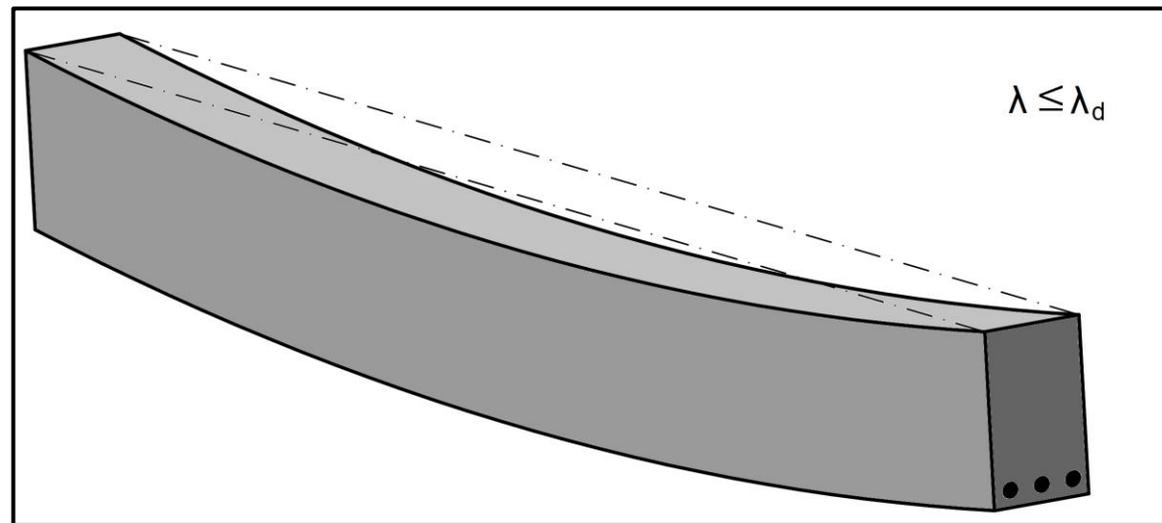
# Span-to-depth ratio assessment

Therefore, **we need to verify** the condition

$$\lambda \leq \lambda_d,$$

where  $\lambda$  is the *Span-to-depth ratio*,

$\lambda_d$  is the *Limiting span-to-depth ratio*.

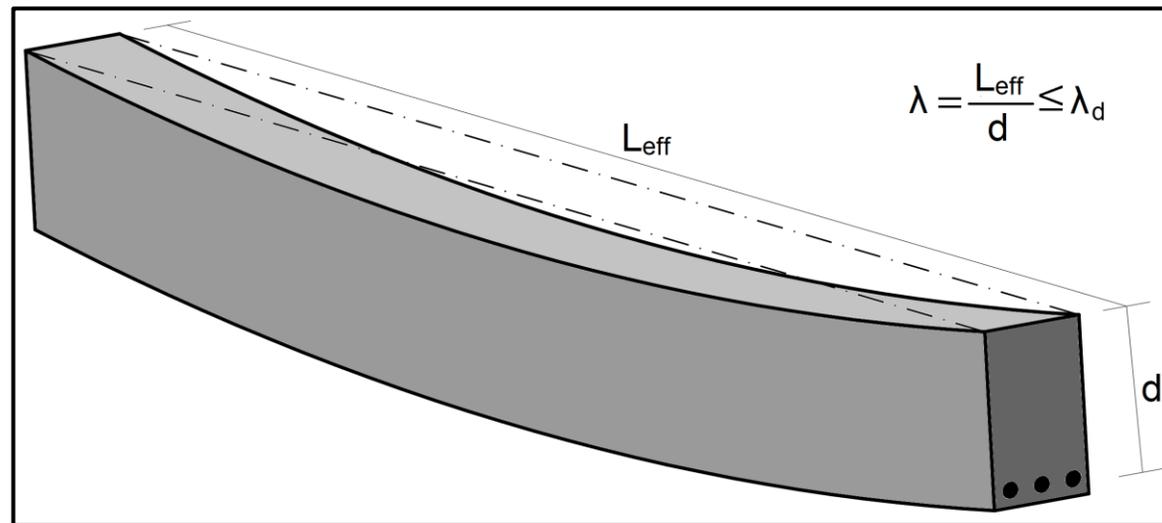


# Span-to-depth ratio $\lambda$

The **Span-to-depth ratio** of a structural member is calculated using the equation

$$\lambda = \frac{l}{d},$$

where  $l$  is the theoretical span (length) of the member\*,  
 $d$  is the effective depth.



# Limiting span-to-depth ratio $\lambda_d$

The **Limiting span-to-depth ratio** is calculated using the equation

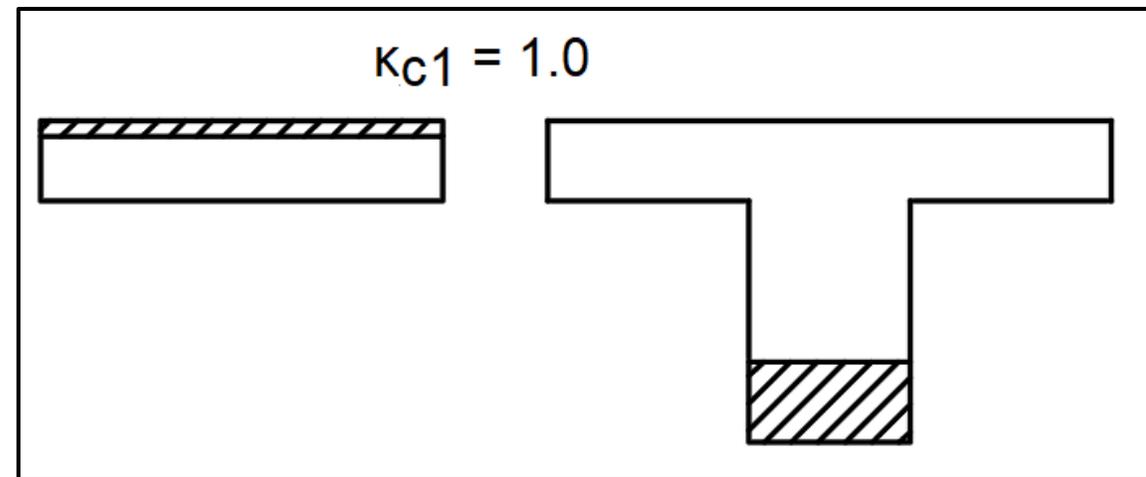
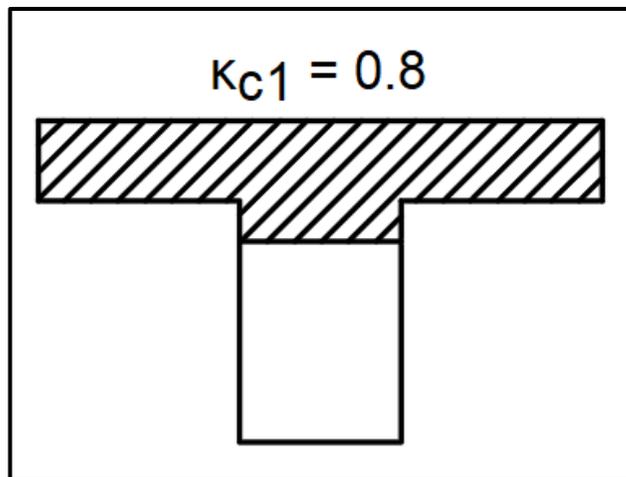
$$\lambda_d = \kappa_{c1}\kappa_{c2}\kappa_{c3}\lambda_{d,tab},$$

where  $\kappa_{c1}$  is the *cross-section shape* factor,  
 $\kappa_{c2}$  is the *span of the element* factor,  
 $\kappa_{c3}$  is the *stress* factor,  
 $\lambda_{d,tab}$  is the tabulated value of the *Basic limiting span-to-depth ratio*.

# Cross-section shape factor $\kappa_{c1}$

The **Cross-section shape factor** is determined based on the cross-section shape:

- $\kappa_{c1} = \mathbf{0.8}$  for a **T-section** – e.g., a beam in the mid-span,
- $\kappa_{c1} = \mathbf{1.0}$  for a **rectangle** – e.g., a slab or a beam above support\*.

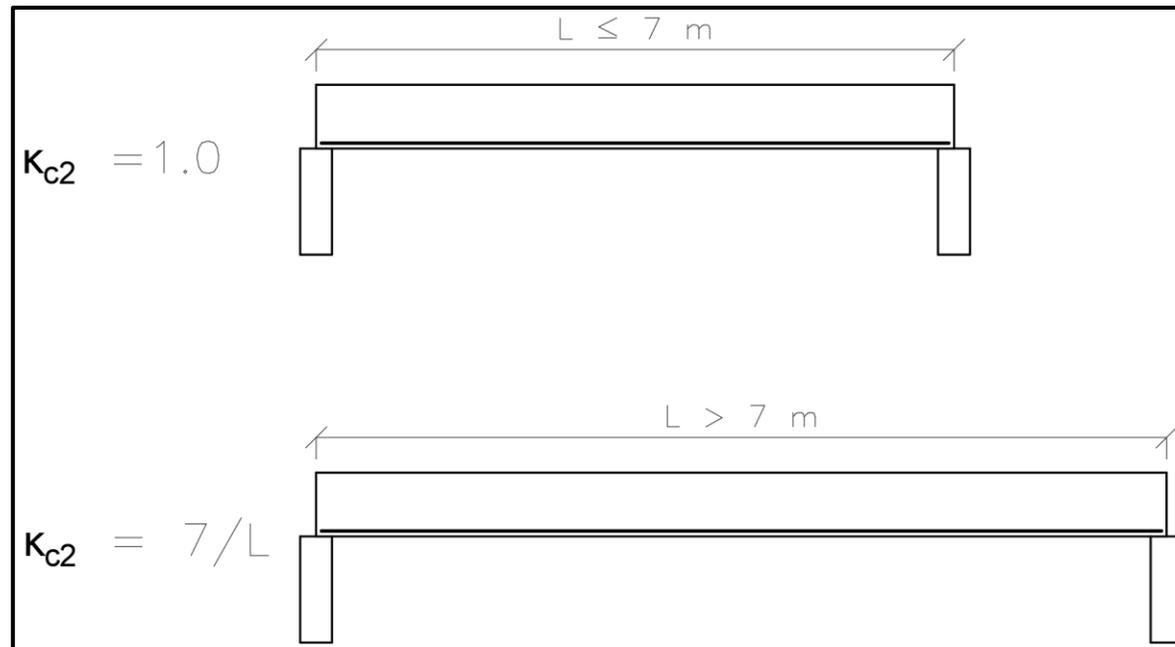


# Span of the element factor $\kappa_{c2}$

The **Span of the element factor** is calculated using the equation

$$\kappa_{c2} = \min\left(1, \frac{7}{l}\right),$$

where  $l$  is the theoretical span of the element in meters.



# Stress factor $\kappa_{c3}$

The **Stress factor** takes into account the actual stress in the reinforcement and is calculated using the equation

$$\kappa_{c3} = \frac{310}{\sigma_s},$$

where  $\sigma_s$  is the maximum\* stress in the reinforcement\*\* (see next slide).

\* In the mid-span of a beam or in the support of a cantilever.

\*\* **This is the stress under a quasi-permanent loading.** (Not the stress used in the load-bearing calculations.)

# Stress factor $\kappa_{c3}$

Unfortunately, we do not know the stress in the reinforcement\*. Fortunately, the standard states that the **Stress factor can be calculated in a simplified** (and conservative) way using the equation

$$\kappa_{c3} = \frac{500}{f_{yk}} \cdot \frac{A_{s,prov}}{A_{s,req}},$$

where  $A_{s,prov}$  is the **designed** area of the **tensile** reinforcement in the cross-section,

$A_{s,req}$  is the **required** area of the **tensile** reinforcement in terms of the acting bending moment,

$f_{yk}$  is the **characteristic** value of the **yield stress** of the reinforcement.

# Basic limiting span-to-depth ratio $\lambda_{d,tab}$

The value of the **Basic limiting span-to-depth ratio** is determined from a table depending on the **strength class** of the concrete, the **tensile reinforcement ratio** ( $A_{s,prov}/bh$ )\*, and the **type of structure**.

Basic limiting span-to-depth ratio for an interior span of a multi-spanned beam.									
$A_{s,prov} / (bh)$	Concrete class								
	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
0.5%	21.9	23.7	25.5	27.8	30.8	34.5	38.6	43.2	48.0
1.5%	18.3	18.9	19.5	20.3	21.0	21.8	22.5	23.3	24.0

\* For values of the reinforcement ratio between 0.5% and 1.5%, the tabulated values need to be interpolated. For a reinforcement ratio of <0.5%, we take a value for 0.5%. For a reinforcement ratio of >1.5%, we take a value for 1.5%.

# Basic limiting span-to-depth ratio $\lambda_{d,tab}$

The value of the **Basic limiting span-to-depth ratio** is determined from a table depending on the **strength class** of the concrete, the **tensile reinforcement ratio** ( $A_{s,prov}/bh$ )\*, and the **type of structure**.

Basic limiting span-to-depth ratio for a cantilever.									
$A_{s,prov} / (bh)$	Concrete class								
	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
0.5%	5.8	6.3	6.8	7.4	8.2	9.2	10.3	11.5	12.8
1.5%	4.9	5.0	5.2	5.4	5.6	5.8	6.0	6.2	6.4

\* For values of the reinforcement ratio between 0.5% and 1.5%, the tabulated values need to be interpolated. For a reinforcement ratio of <0.5%, we take a value for 0.5%. For a reinforcement ratio of >1.5%, we take a value for 1.5%.

# Basic limiting span-to-depth ratio $\lambda_{d,tab}$

The value of the **Basic limiting span-to-depth ratio** is determined from a table depending on the **strength class** of the concrete, the **tensile reinforcement ratio** ( $A_{s,prov}/bh$ )\*, and the **type of structure**.

Basic limiting span-to-depth ratio for end span of a multi-spaned beam.									
$A_{s,prov} / (bh)$	Concrete class								
	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
0.5%	19.0	20.5	22.1	24.1	26.7	29.9	33.5	37.4	41.6
1.5%	15.9	16.4	16.9	17.6	18.2	18.9	19.5	20.2	20.8

\* For values of the reinforcement ratio between 0.5% and 1.5%, the tabulated values need to be interpolated. For a reinforcement ratio of <0.5%, we take a value for 0.5%. For a reinforcement ratio of >1.5%, we take a value for 1.5%.

# Basic limiting span-to-depth ratio $\lambda_{d,tab}$

The value of the **Basic limiting span-to-depth ratio** is determined from a table depending on the **strength class** of the concrete, the **tensile reinforcement ratio** ( $A_{s,prov}/bh$ )\*, and the **type of structure**.

Basic limiting span-to-depth ratio for interior span of a simply-supported beam.									
$A_{s,prov} / (bh)$	Concrete class								
	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
0.5%	14.6	15.8	17.0	18.5	20.5	23.0	25.8	28.8	32.0
1.5%	12.2	12.6	13.0	13.5	14.0	14.5	15.0	15.5	16.0

\* For values of the reinforcement ratio between 0.5% and 1.5%, the tabulated values need to be interpolated. For a reinforcement ratio of <0.5%, we take a value for 0.5%. For a reinforcement ratio of >1.5%, we take a value for 1.5%.

# Basic limiting span-to-depth ratio $\lambda_{d,tab}$

The value of the **Basic limiting span-to-depth ratio** is determined from a table depending on the **strength class** of the concrete, the **tensile reinforcement ratio** ( $A_{s,prov}/bh$ )\*, and the **type of structure**.

Basic limiting span-to-depth ratio for interior span of a locally supported slab.									
$A_{s,prov} / (bh)$	Concrete class								
	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
0.5%	17.5	19.0	20.4	22.2	24.6	27.6	30.9	34.5	38.4
1.5%	14.6	15.1	15.6	16.2	16.8	17.4	18.0	18.6	19.2

\* For values of the reinforcement ratio between 0.5% and 1.5%, the tabulated values need to be interpolated. For a reinforcement ratio of <0.5%, we take a value for 0.5%. For a reinforcement ratio of >1.5%, we take a value for 1.5%.

# Span-to-depth ratio assessment

Now, when we know the Span-to-depth ratio as well as the Limiting span-to-depth ratio, we can verify the condition

$$\lambda \leq \lambda_d,$$

where  $\lambda$  is the Span-to-depth ratio,

$\lambda_d$  is the Limiting span-to-depth ratio.

# Span-to-depth ratio assessment

If **the Span-to-depth ratio condition is not satisfied**, it does not necessarily mean that the design is not correct!

It just means that this simplified condition is not satisfied, and that **the deflection must be assessed by calculating and assessing the deflection directly** (see the beginning of this presentation).

Thank you for your attention