



decivil universidade de aveiro
departamento de engenharia civil



PhD: FIRE DESIGN OF UNIFORM AND TAPERED CLASS 4 STEEL MEMBERS

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Scientific Supervision Team:

Paulo Vila Real
University of Aveiro

Nuno Lopes
University of Aveiro

Bin Zhao
CTICM



Point of Situation

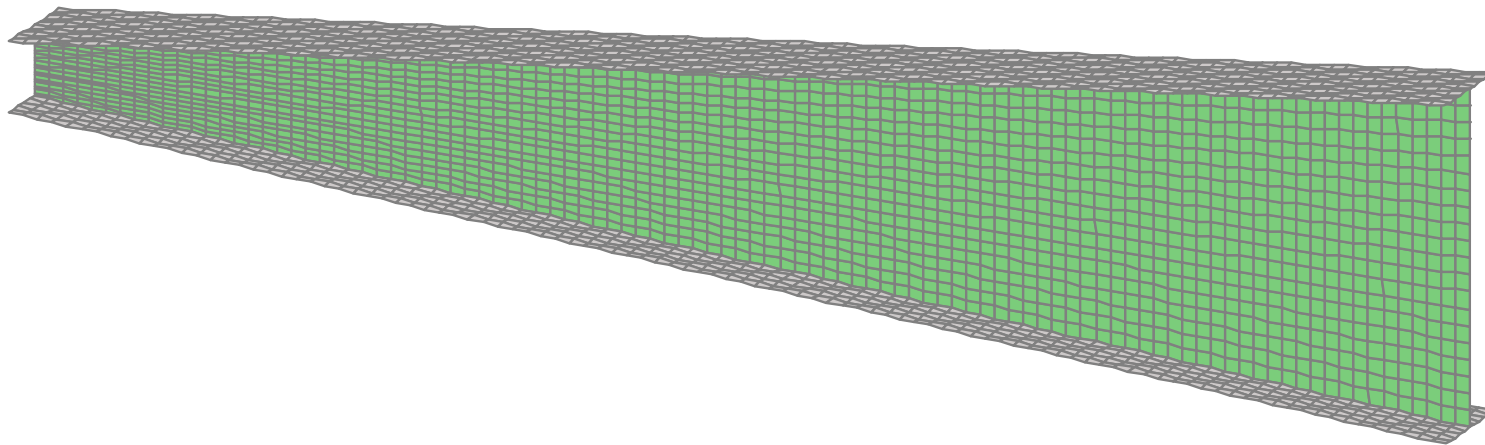
- My PhD started in February 2012





Motivation

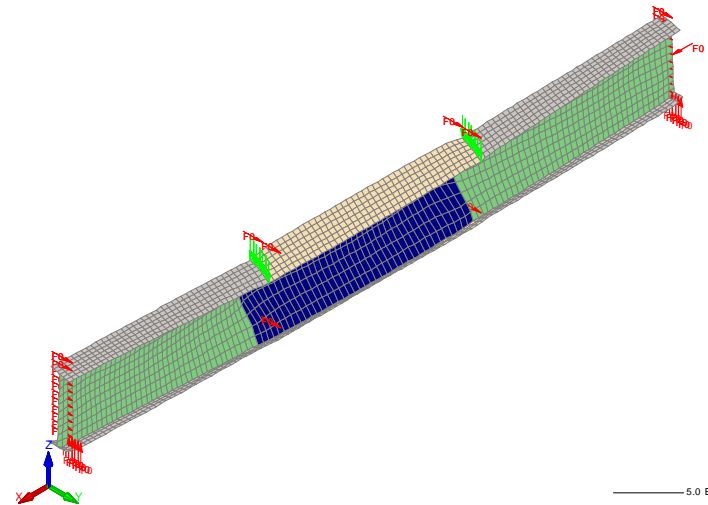
- H or I steel profiles with class 4 cross-section are widely used
- Fire design rules from the Eurocode for these elements have demonstrated to be very conservative



- For tapered members it is not clear if normal temperature design rules can be adapted for fire design



FIDESC4 - FIRE DESIGN OF STEEL MEMBERS WITH WELDED OR HOT-ROLLED CLASS 4 CROSS-SECTION RFCR-CT-2011-00030, 2011-2014



Partnership:

- CTICM(Coordinator, France)
- LINDAB (Luxembourg, Steel Construction Company)
- Tecnalía (Spain, LABEIN)
- **University of Aveiro (Portugal)**
- Czech Technical University in Prague (Czech)
- University of Liège (Belgium)
- Desmo (Czech, Steel Construction Company)



WP1 Design of fire tests, benchmark study and definition of numerical parametric studies – Coord. CTICM

WP2 Simple bending – Coord. CTICM

WP3 LTB under bending – Coord. CTU

WP4 Axial compression – Coord. ULG

WP5 Combined bending and buckling – Coord. UnivAveiro (UA)

WP6 Development of user-friendly software to apply simple design rules – Coord. UA

WP7 Global structural analysis – Coord. CTICM



FIRE DESIGN OF UNIFORM AND TAPERED CLASS 4 STEEL MEMBERS

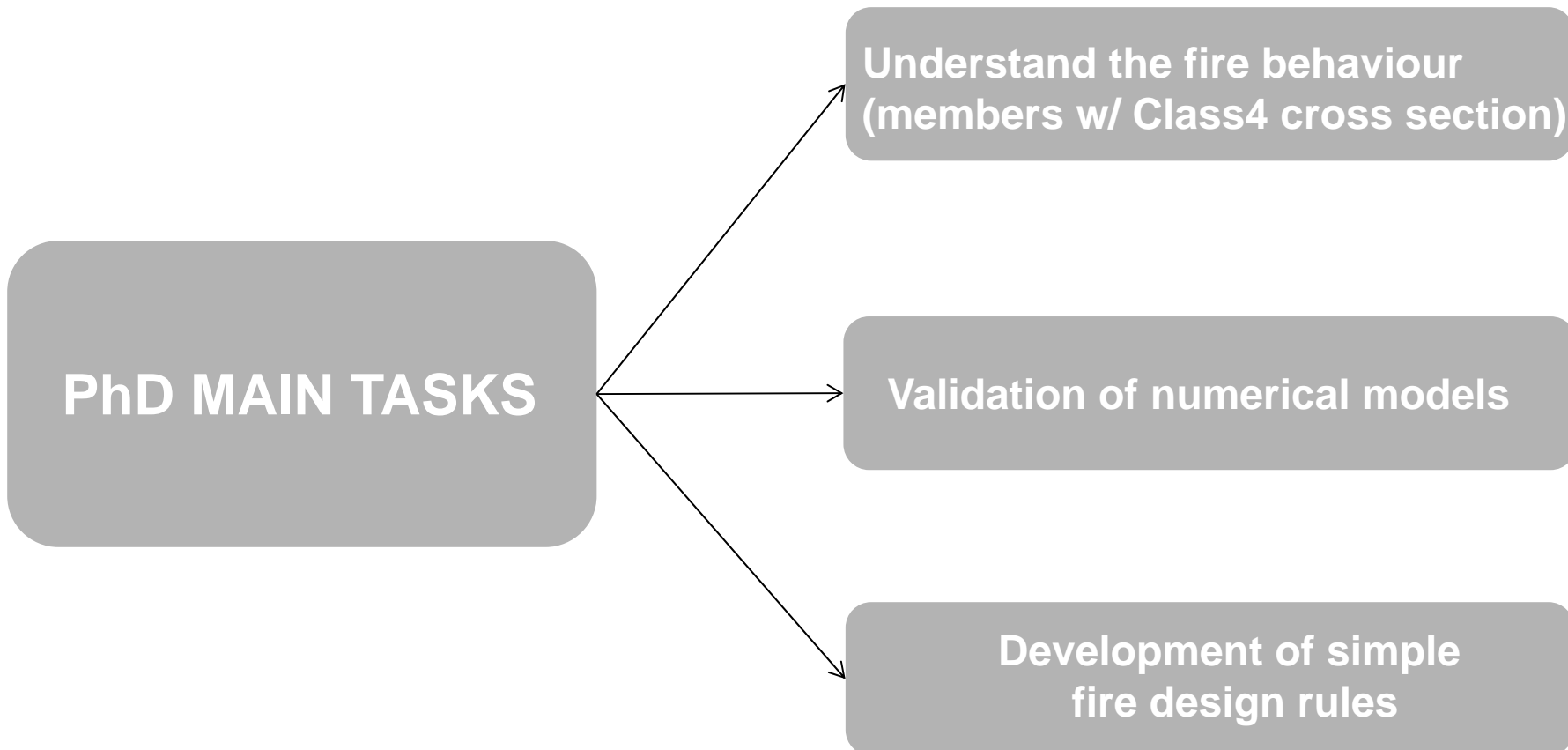
objectives:

Study and to **develop simple design rules** for fire design of uniform and tapered steel members with welded or hot-rolled H or I shape class 4 cross-sections

Particularity for **beams** and **beam-columns** as close as possible to the principles of design rules of Eurocode 3 at room temperature



Main Tasks





PhD Working Programme

- A** **Bibliographic research.**
- B** **Study of lateral-torsional buckling of unrestrained class 4 beams.**
- C** **Validation of the used numerical model on the basis of experimental tests (from FIDESC4...).**
- D** **Validation of the formulae currently available in Part 1-1 of the Eurocode 3 for the design and stability check of steel tapered members at room temperature and the adaptation to fire situation.**
- E** **Parametric study.**
- F** **Development of simple fire design rules.**
- G** **Final writings of the PhD Thesis.**



PhD Working Programme

A Bibliographic research.

B Study of lateral-torsional buckling of unrestrained class 4 beams.

C Validation of the used numerical model on the basis of experimental tests (from FIDESC4...).

D Validation of the formulae currently available in Part 1-1 of the Eurocode 3 for the design and stability check of steel tapered members at room temperature and adaptation to fire situation.

E Parametric study.

F Development of simple fire design rules.

G Final writings of the PhD Thesis.



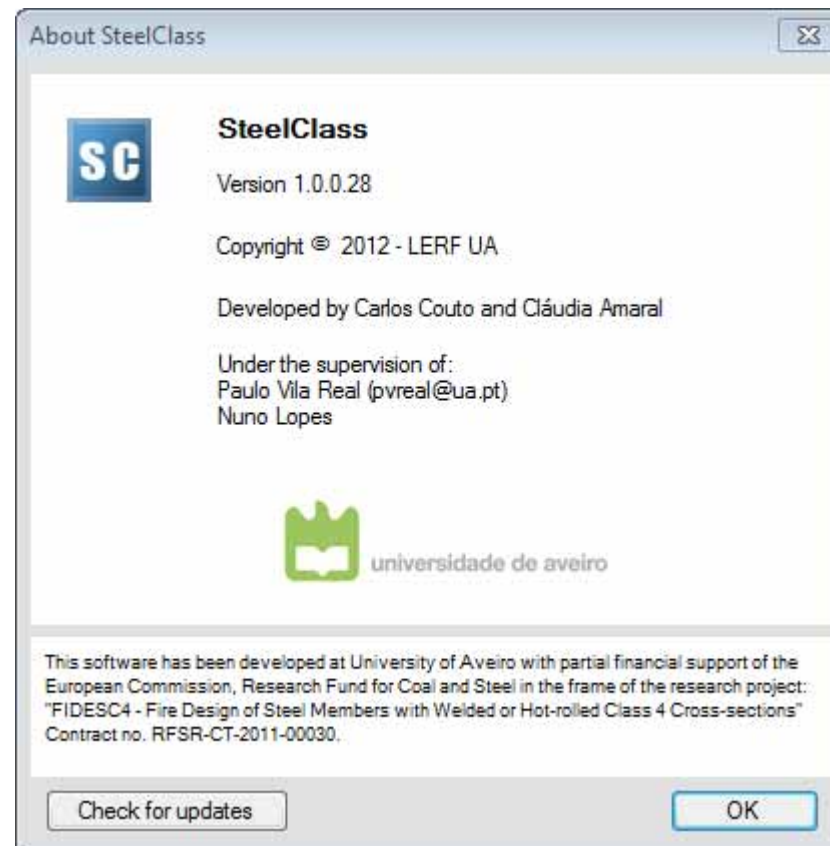
Software developed

1. Development of software



Software developed

Development of SteelClass, a software for cross section classification...



... and properties!



Software developed

User Profiles Manager

Edit Profile

Profile name

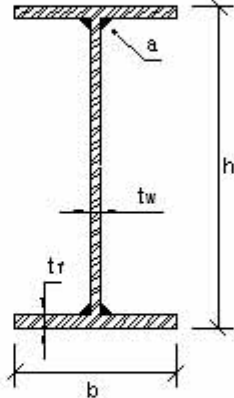
$h =$ mm

$b =$ mm

$t_w =$ mm

$t_f =$ mm

$a =$ mm



Profile fabrication

Rolled profile Welded profile

Ok Cancel



Software developed

SteelClass - Cross Section Classification

File Edit View Help

Design situation:
fire design

Profile Serie:
Other

Profile Name:
FIDESC4

Steel Grade:
S355

Enter the forces for combined bending and axial classification:
 Compression
 Tension

N_{Ed} 0 kN

**FIDESC4
Gross Section**

460

5

150

4

[mm]

Under Compression
flange is Class 4
web is Class 4

Under Bending about y-y
flange is Class 4
web is Class 4

Under Bending about z-z
flange is Class 4

Gross Section Effective N Effective My Effective Mz More Details Classification Properties



Software developed

SteelClass - Cross Section Classification

File Edit View Help

Design situation:
fire design

Profile Serie:
Other

Profile Name:
FIDESC4

Steel Grade:
S355

Enter the forces for combined bending and axial classification:
 Compression
 Tension
 N_{Ed} 100 kN

**FIDESC4
Gross Section**

460

5

150

[mm]

Under Compression
flange is Class 4
web is Class 4

Under Bending about y-y
flange is Class 4
web is Class 4

Under Bending about z-z
flange is Class 4

Under Bending y-y w/Compression
flange is Class 4
web is Class 4

Under Bending z-z w/Compression
flange is Class 4
web is Class 4

Gross Section Effective N Effective My Effective Mz More Details Classification Properties



Software developed

SteelClass - Cross Section Classification

File Edit View Help

Design situation:
fire design

Profile Serie:
Other

Profile Name:
FIDESC4

Steel Grade:
S355

Enter the forces for combined bending and axial classification:
 Compression
 Tension
 N_{Ed} 100 kN

FIDESC4
Bending about y-y

460

150

12.0 126.0 12.0

5

80.2

33.2

126.2

Z_g

210.4

4

[mm]

h	=	460	mm
b	=	150	mm
t_w	=	4	mm
t_f	=	5	mm
a	=	0	mm
A	=	3300	mm ²
A_{eff}	=	1932	mm ²
A_{vz}	=	1820	mm ²
A_{vy}	=	1500	mm ²
I_T	=	22.1E3	mm ⁴
I_W	=	14.6E10	mm ⁶
I_y	=	108012500	mm ⁴
$I_{eff,y}$	=	98950946	mm ⁴
$W_{pl,y}$	=	543750	mm ³
$W_{el,y}$	=	469620	mm ³
$W_{eff,y,min}$	=	404611	mm ³
i_y	=	181	mm
I_z	=	2814900	mm ⁴
$I_{eff,z}$	=	2517639	mm ⁴
$W_{pl,z}$	=	58050	mm ³
$W_{el,z}$	=	37532	mm ³
$W_{eff,z,min}$	=	33019	mm ³
i_z	=	29	mm

Gross Section Effective N Effective My Effective Mz More Details Classification Properties



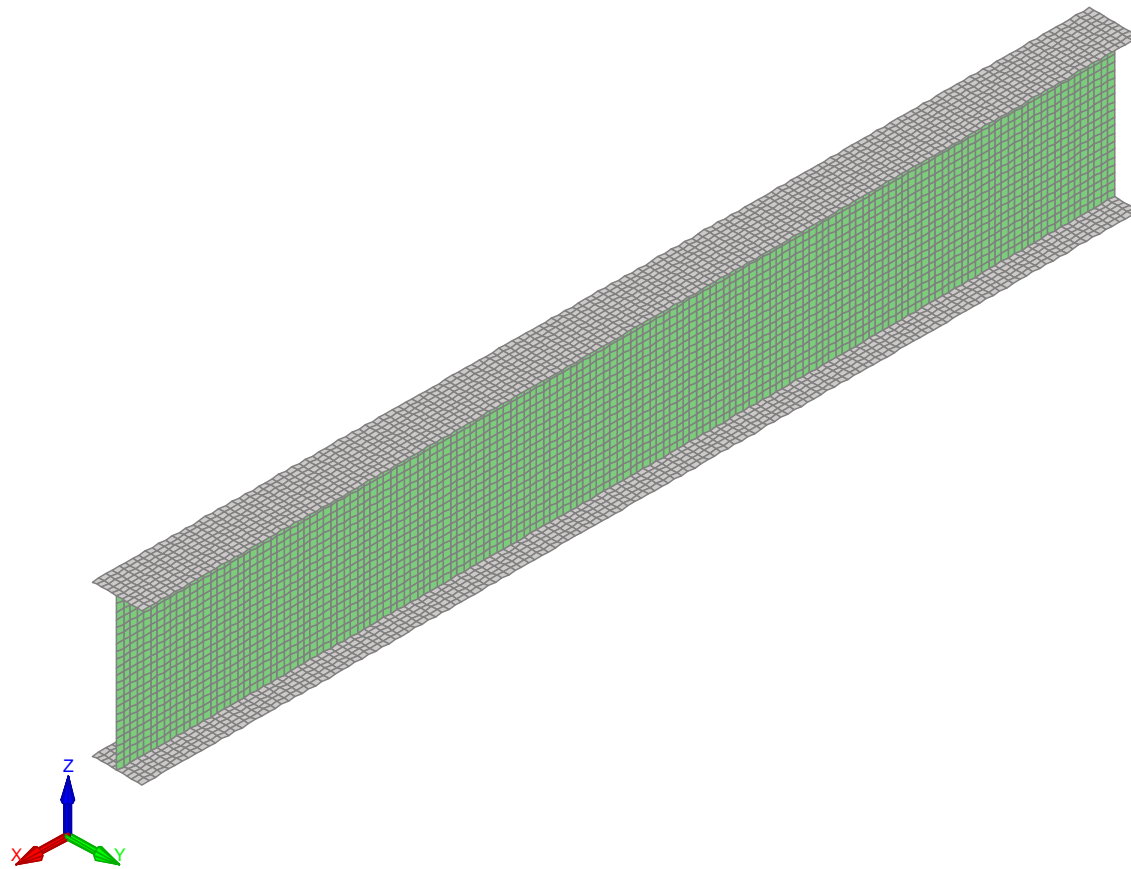
Moving next...

2. LTB Study



My LTB shell finite element model

Mesh size: 25 mm (approx.)



Diamond 2011.a.2 for SAFIR

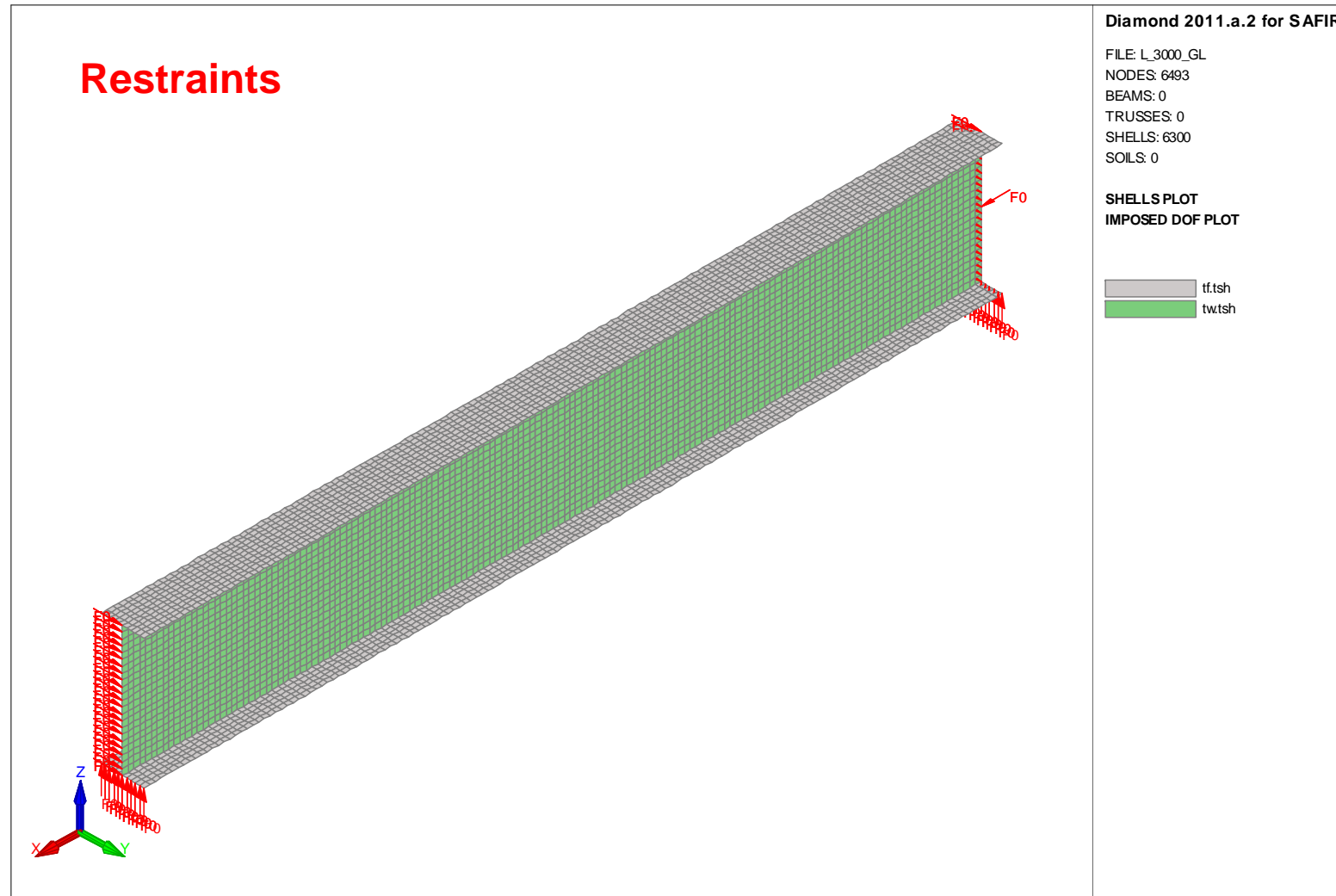
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SHELLS: 6300
SOILS: 0

SHELLS PLOT

tf.tsh
tw.tsh

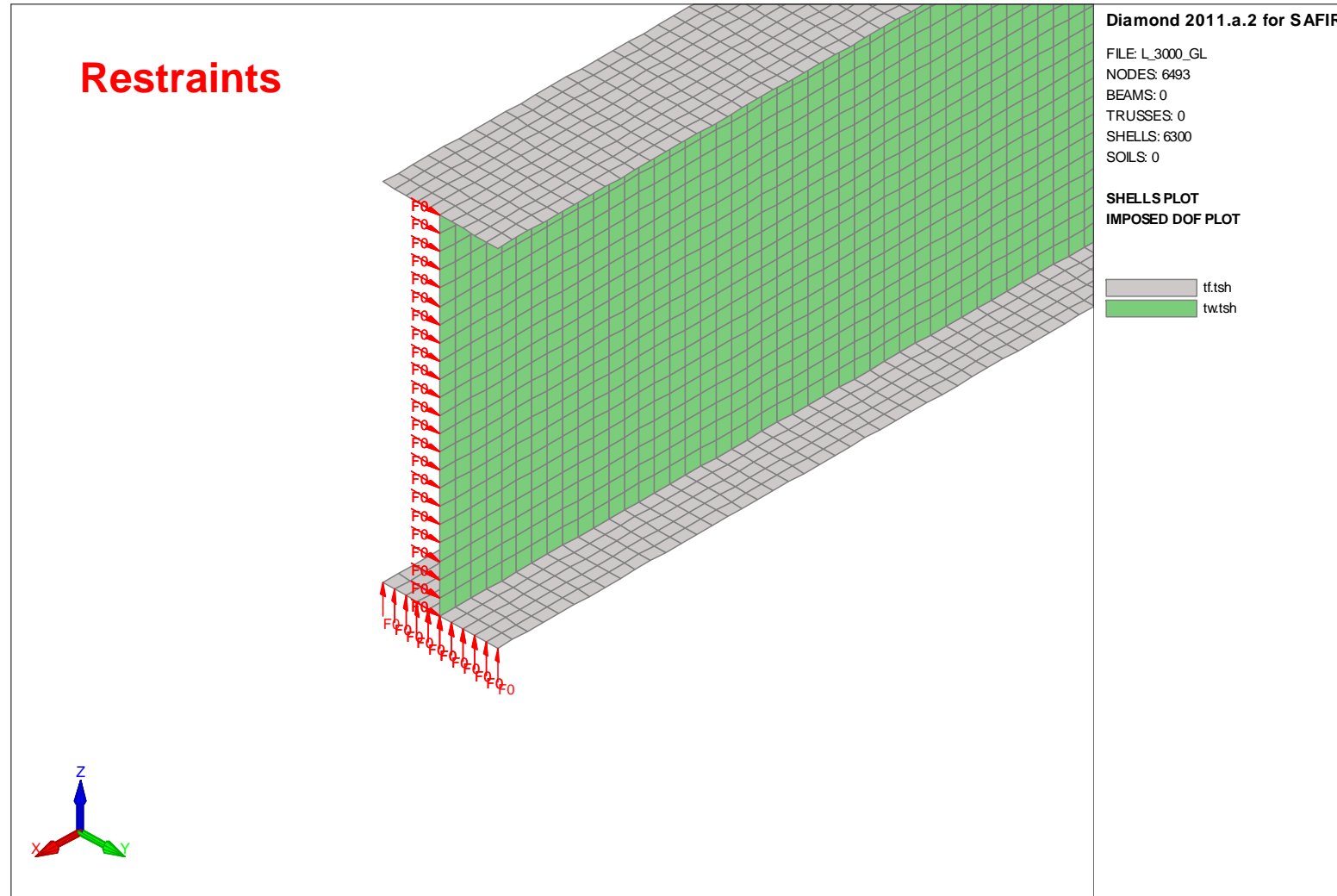


My LTB shell finite element model



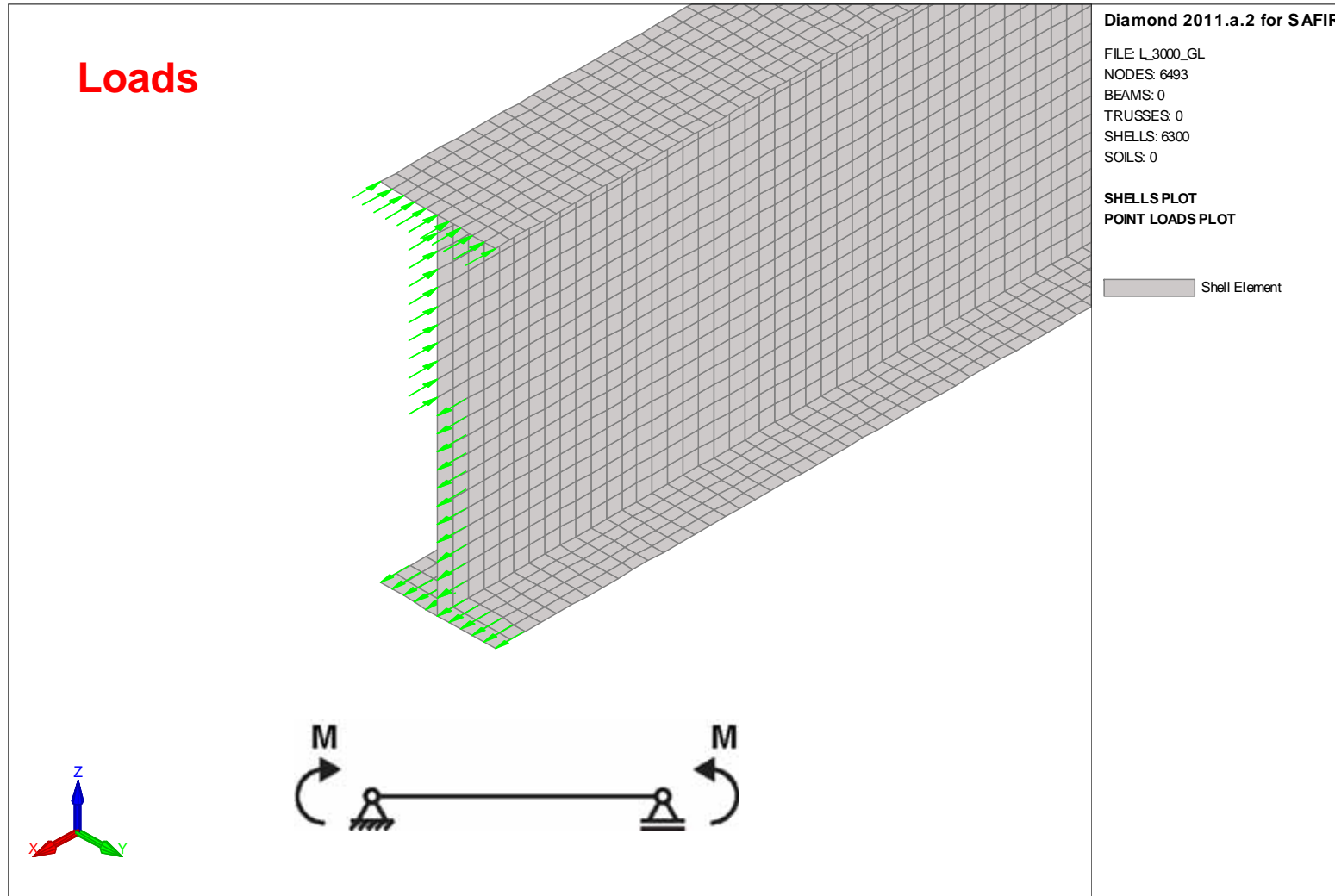


My LTB shell finite element model





My LTB shell finite element model



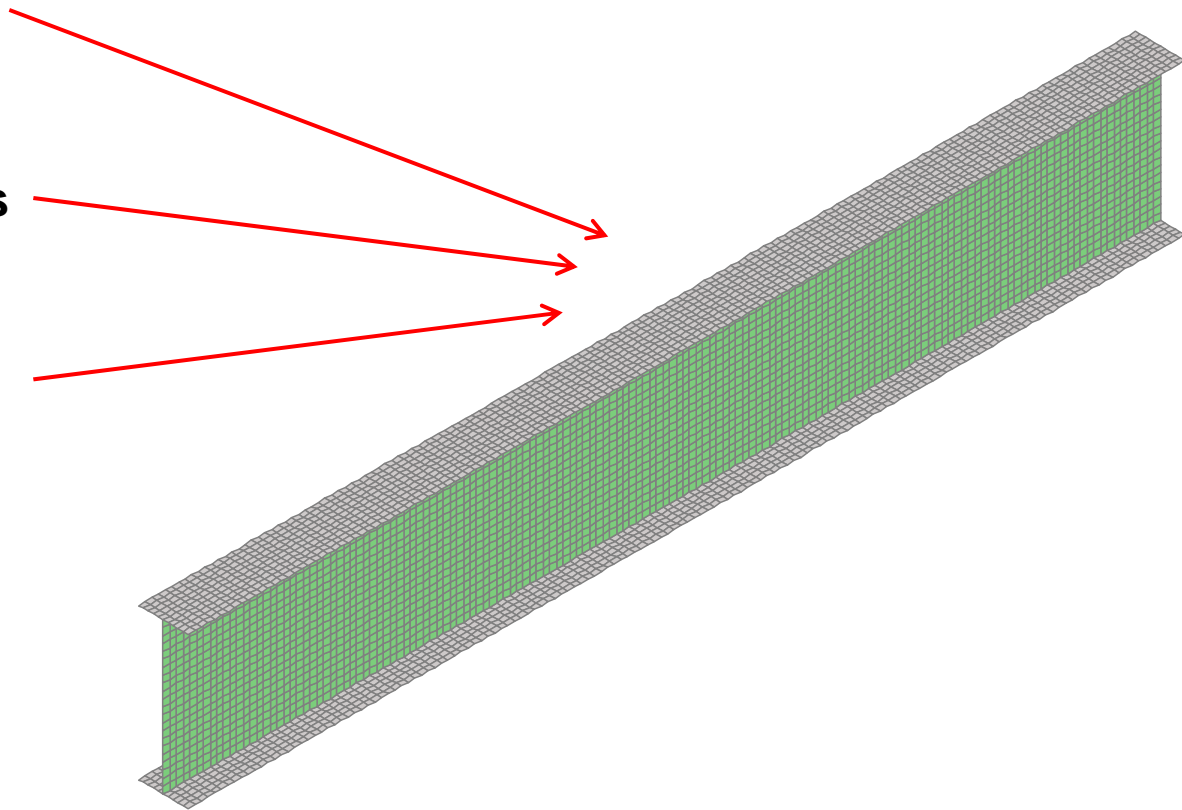


My LTB shell finite element model

Local imperfections

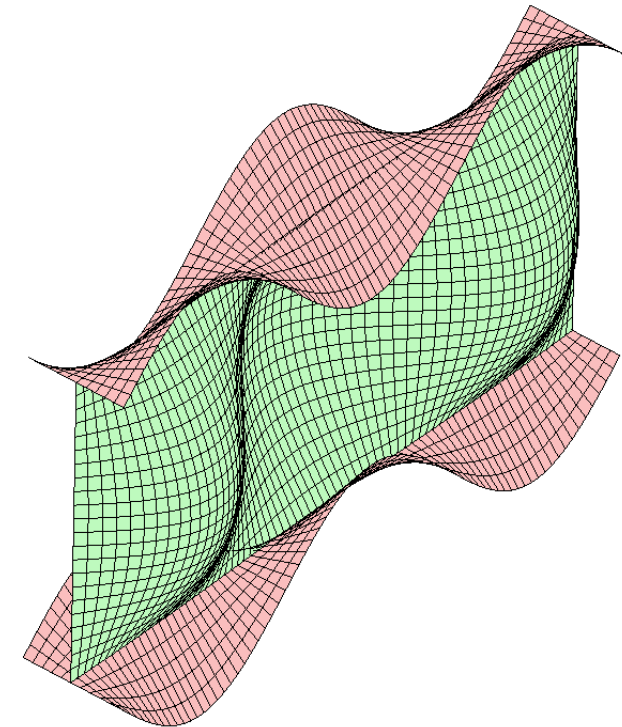
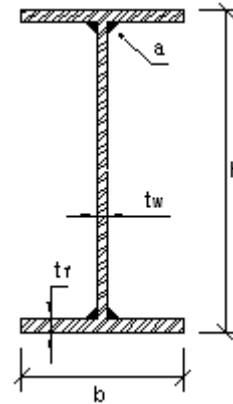
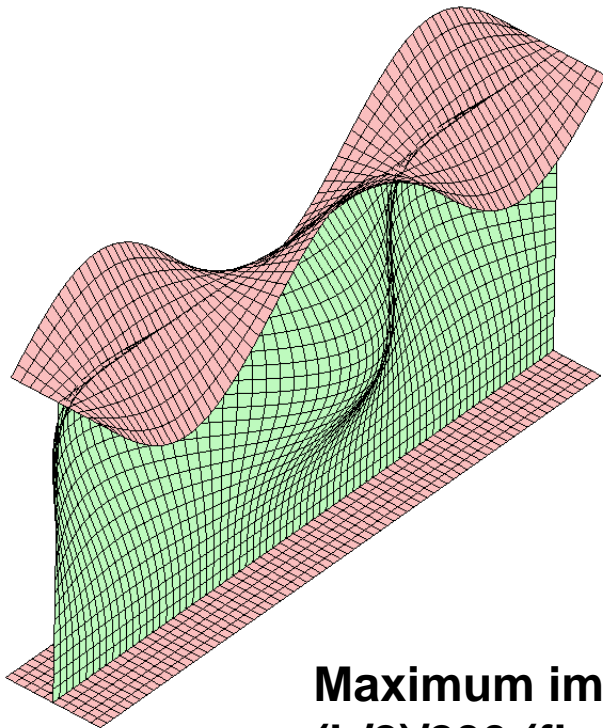
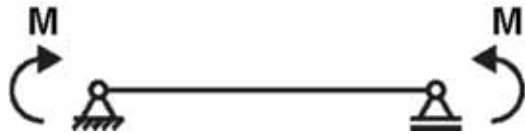
Global imperfections

Residual stresses





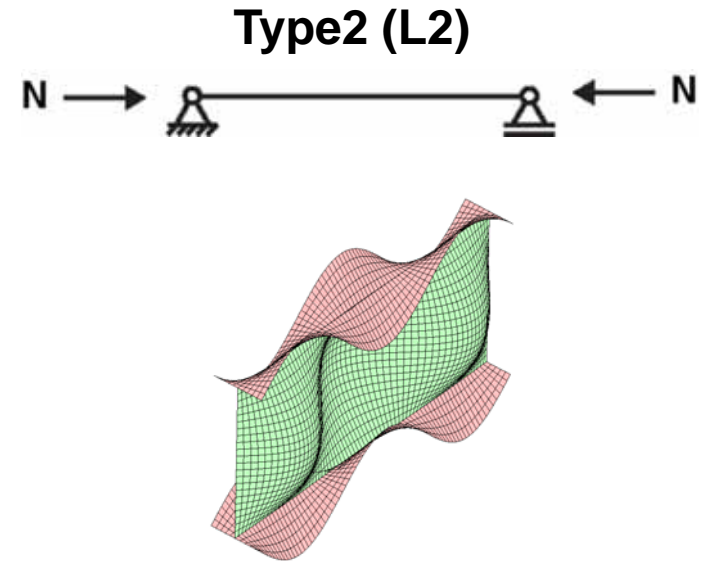
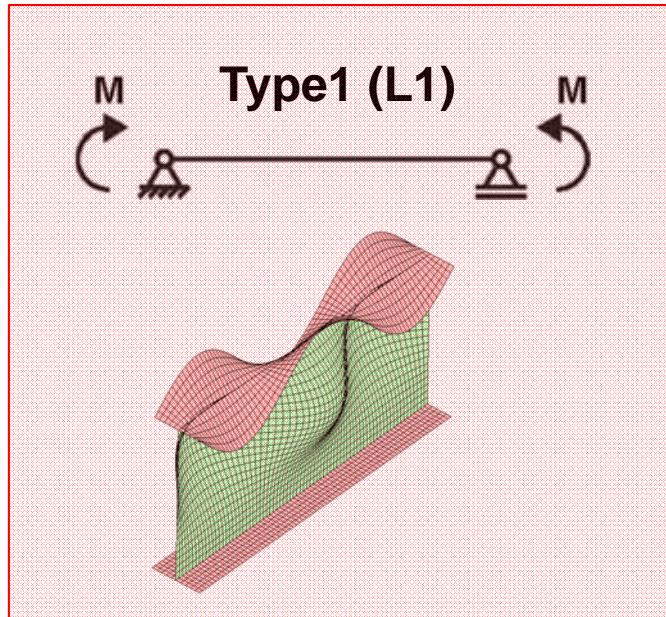
Local Imperfections



Maximum imperfection amplitude:
 $(b/2)/200$ (flange)
 $(h-2 t_f)/200$ (web)



Local Imperfections



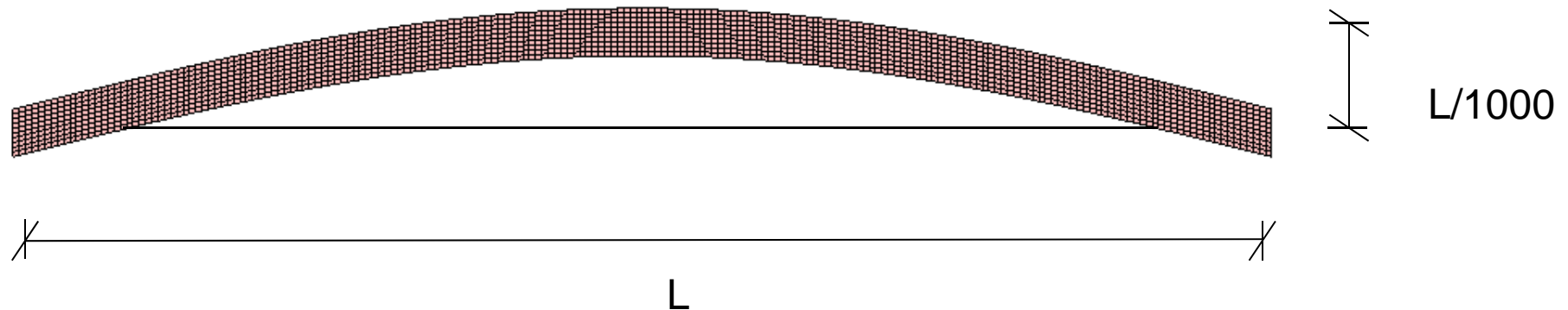
T (°C)	L1	L2	L1/L2	Dif (%)
20	1094.85750	1098.43750	1.00327	-0.33
400	1221.87000	1225.07000	1.00262	-0.26
500	1104.12000	1104.78750	1.00060	-0.06
600	830.55750	831.74250	1.00143	-0.14

T (°C)	No Imp	L1	No Imp/L1	Dif (%)
20	1402.475	1094.85750	0.78066	21.93
400	1339.66	1221.87000	0.91207	8.79
500	1204.81	1104.12000	0.91643	8.36
600	919.35	830.55750	0.90342	9.66



Global Imperfections

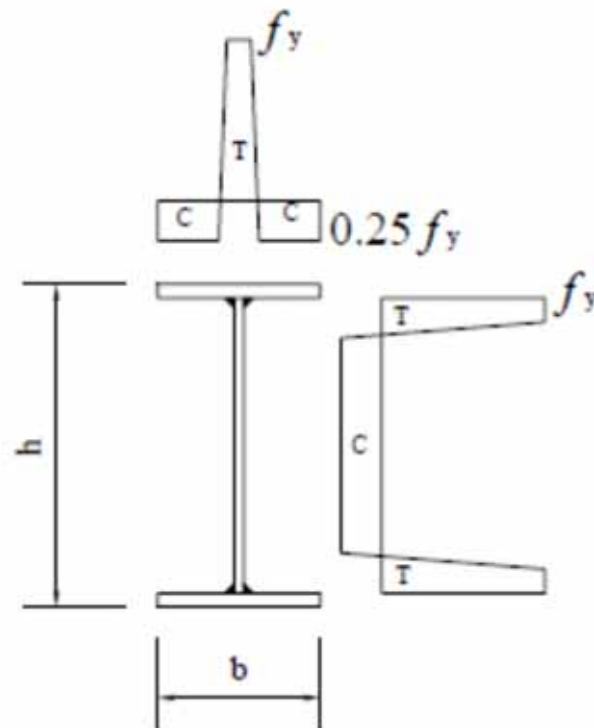
A global imperfection of $L/1000$ was considered.





Residual Stress

Pattern of the residual stress considered in the shell elements (welded profile):

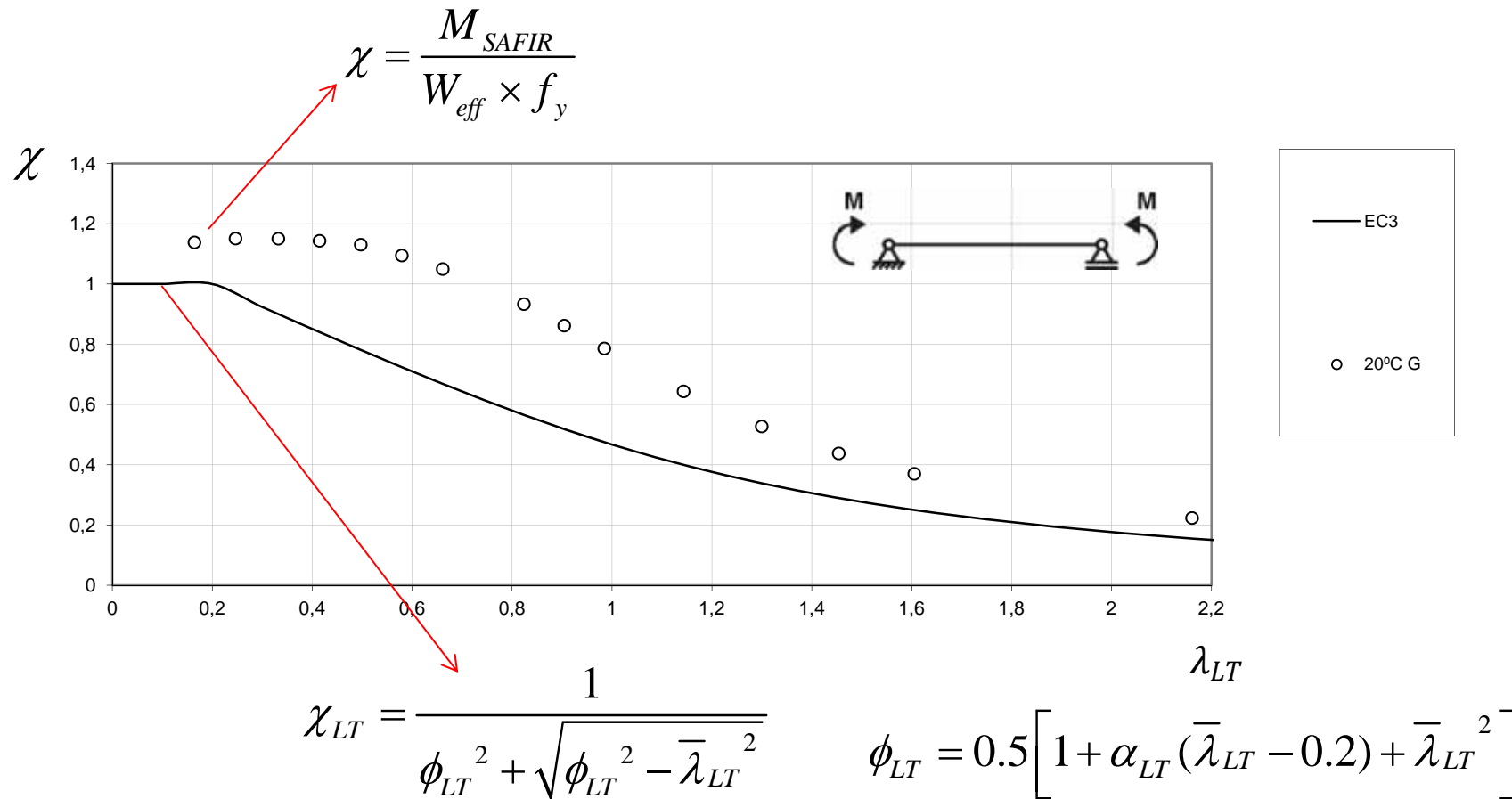


Problem!

If $f_y=355$ MPa SAFIR model doesn't run. If $f_y=340$ MPa for example, it's okay!



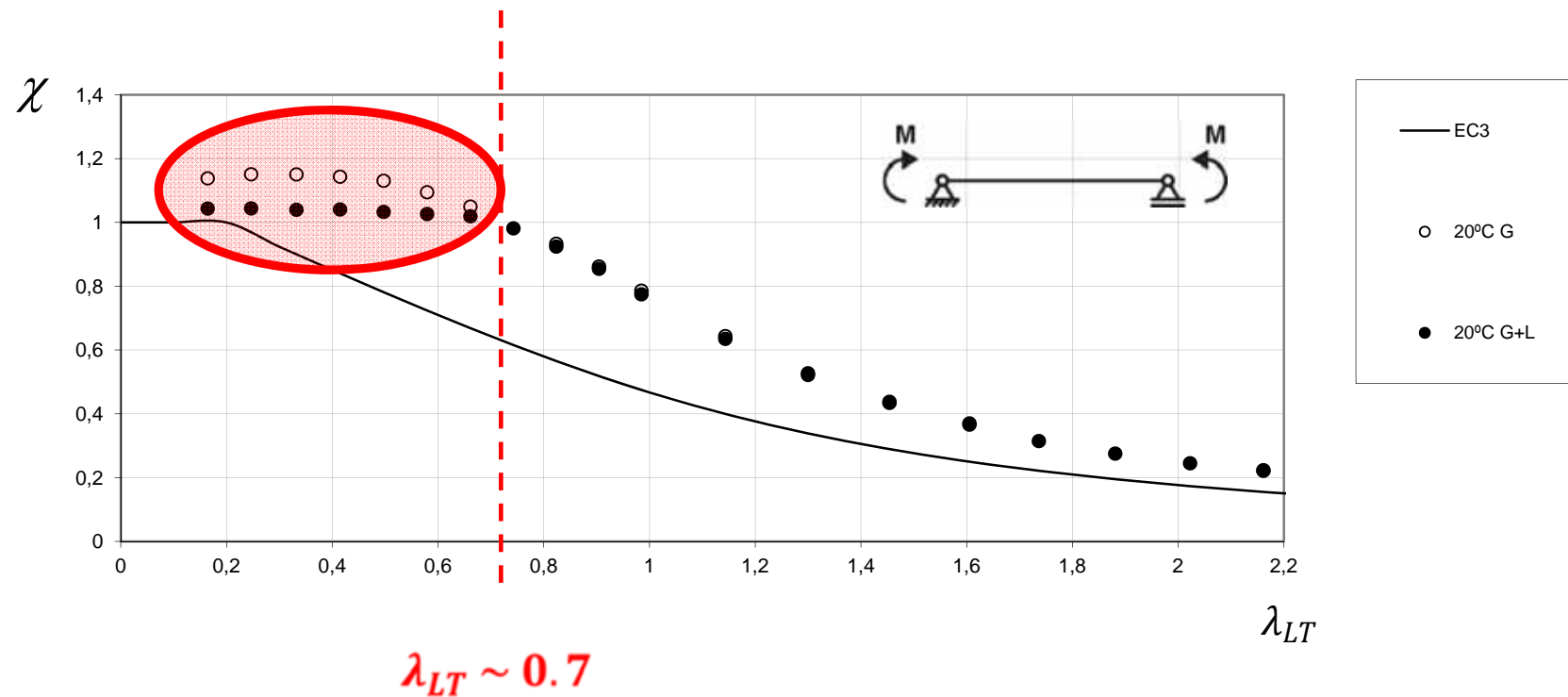
Room temperature results with only **Global Imperfections**





Results

Room temperature results with **Global Imperfections + Local Imperfections**

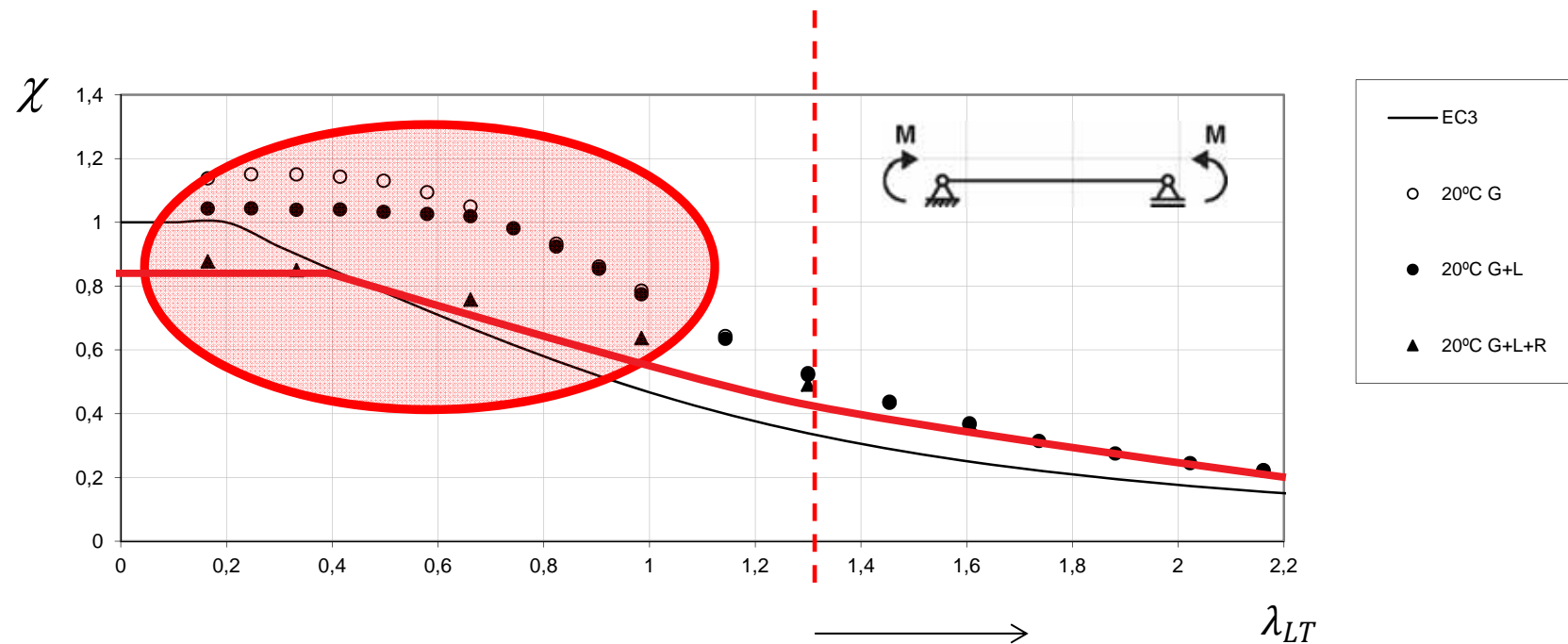




Results

Room temperature results with:

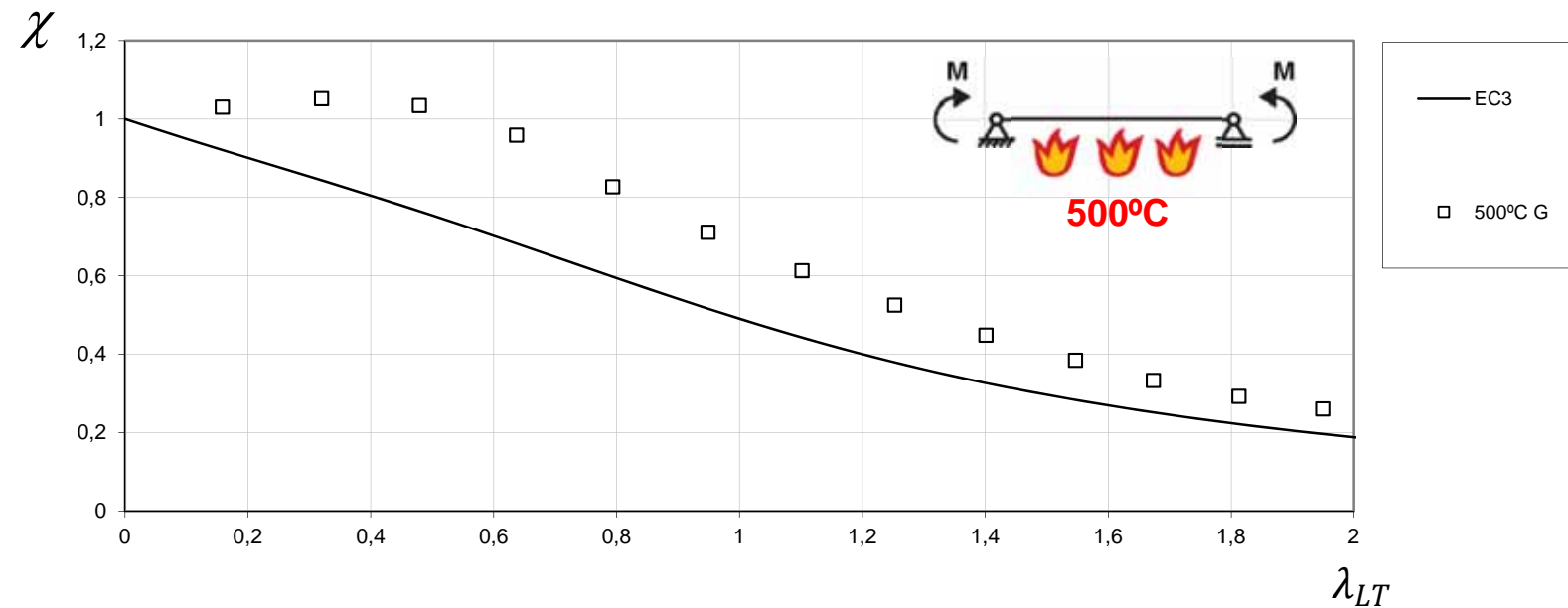
Global Imperfections + Local Imperfections + Residual stresses





Results

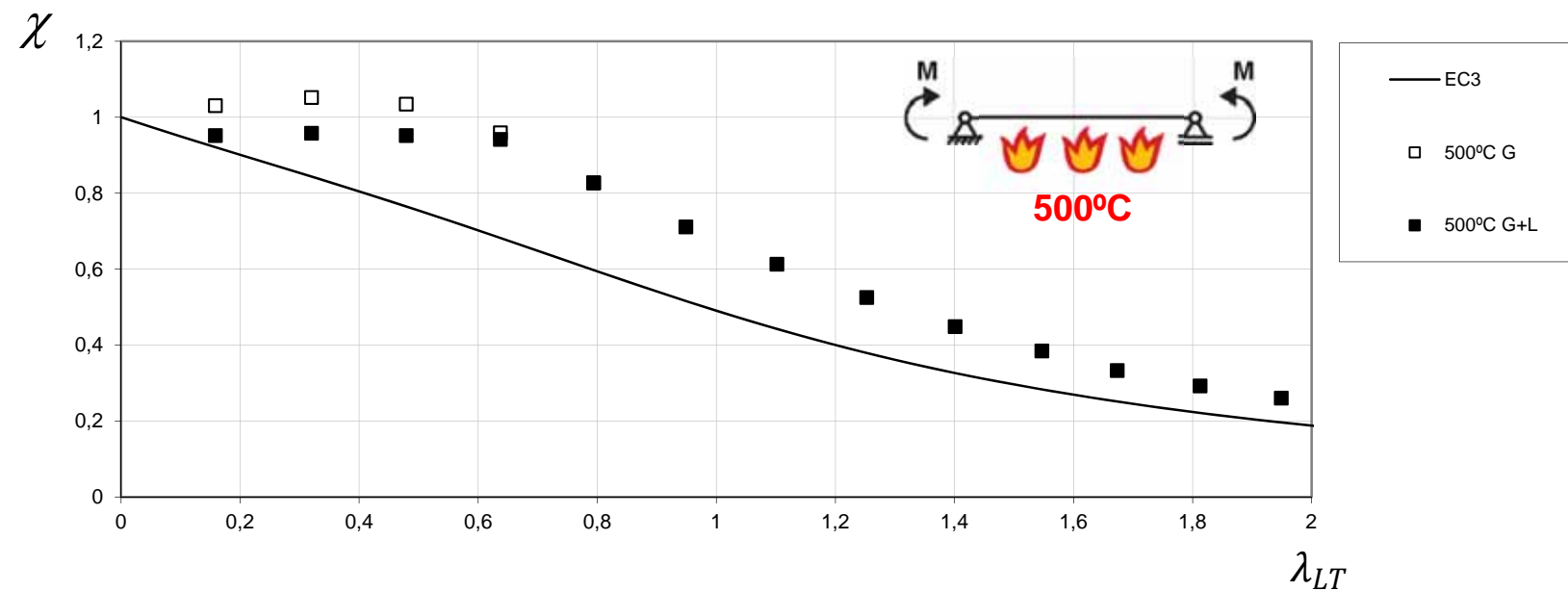
Results at 500°C with **Global Imperfections**





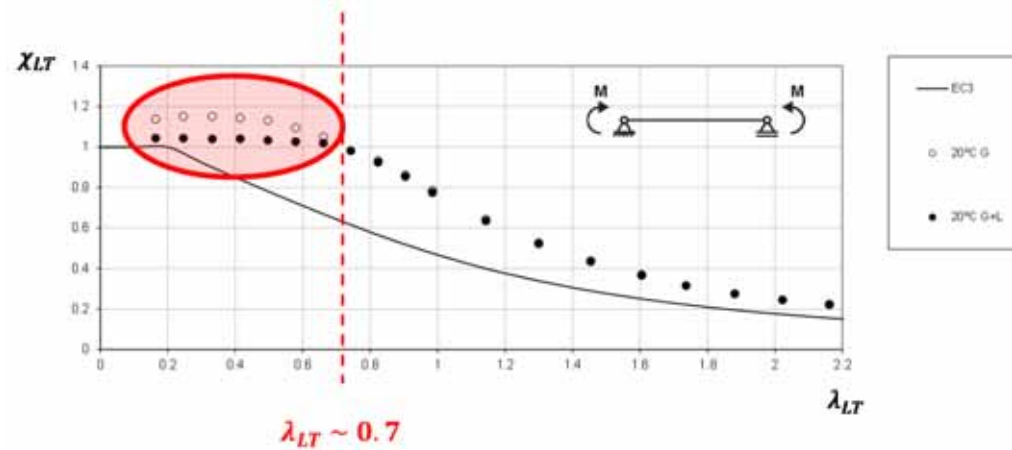
Results

Results at 500°C with **Global Imperfections + Local Imperfections**



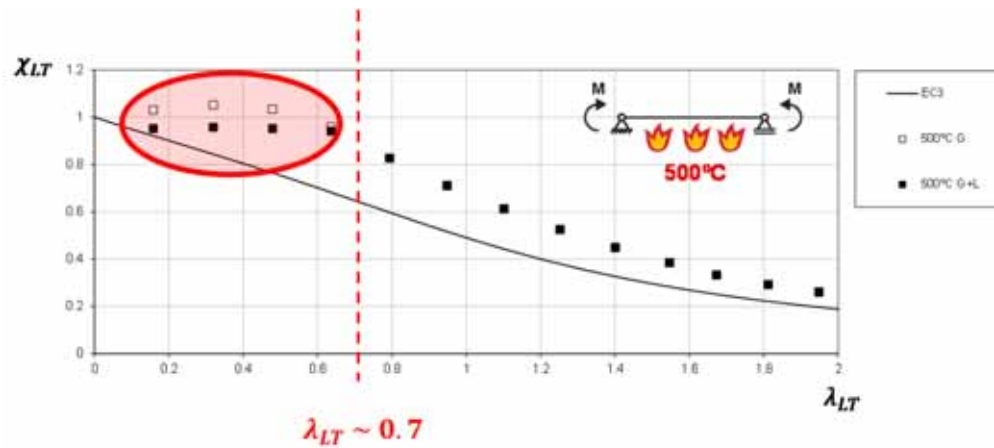


Results



For Class 4:

$$\varepsilon_{\theta} \approx \varepsilon_{20^{\circ}\text{C}}$$



$$\bar{\lambda}_{p,\theta} \approx \bar{\lambda}_{20^{\circ}\text{C}}$$



Results

$$\begin{aligned}\bar{\lambda}_p &= \sqrt{\frac{f_y}{\sigma_{cr}}} = \sqrt{\frac{f_y}{k_\sigma \frac{\pi^2 E t^2}{12(1-\nu^2)b^2}}} = \frac{b/t}{\sqrt{k_\sigma}} \frac{1}{\sqrt{\frac{\pi^2}{12(1-\nu^2)} \frac{E}{f_y}}} = \\ &= \frac{b/t}{\sqrt{k_\sigma} \sqrt{\frac{\pi^2}{12(1-\nu^2)} \sqrt{\frac{210000}{235}} \sqrt{\frac{235}{f_y}} \sqrt{\frac{E}{210000}}}} = \\ &= \frac{b/t}{28.4\sqrt{k_\sigma}} \frac{1}{\sqrt{\frac{235}{f_y}} \sqrt{\frac{E}{210000}}} = \frac{b/t}{28.4\sqrt{k_\sigma}} \frac{1}{\varepsilon} = \frac{b/t}{28.4\varepsilon\sqrt{k_\sigma}}\end{aligned}$$

$$\varepsilon = \sqrt{\frac{235}{f_y}} \sqrt{\frac{E}{210000}} \quad \text{with } f_y \text{ and } E \text{ in MPa}$$

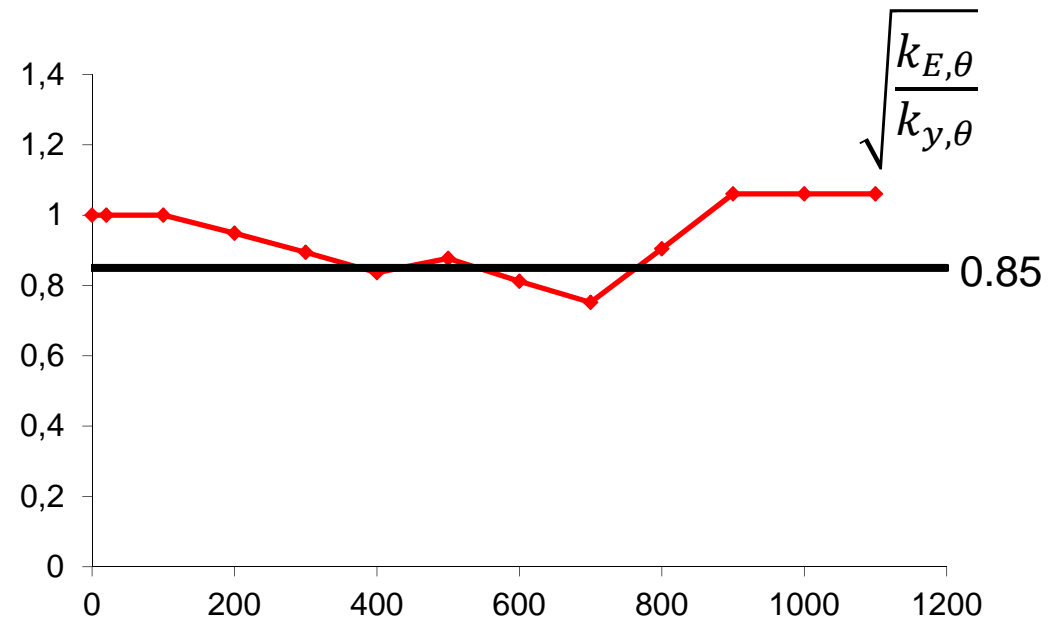


Results

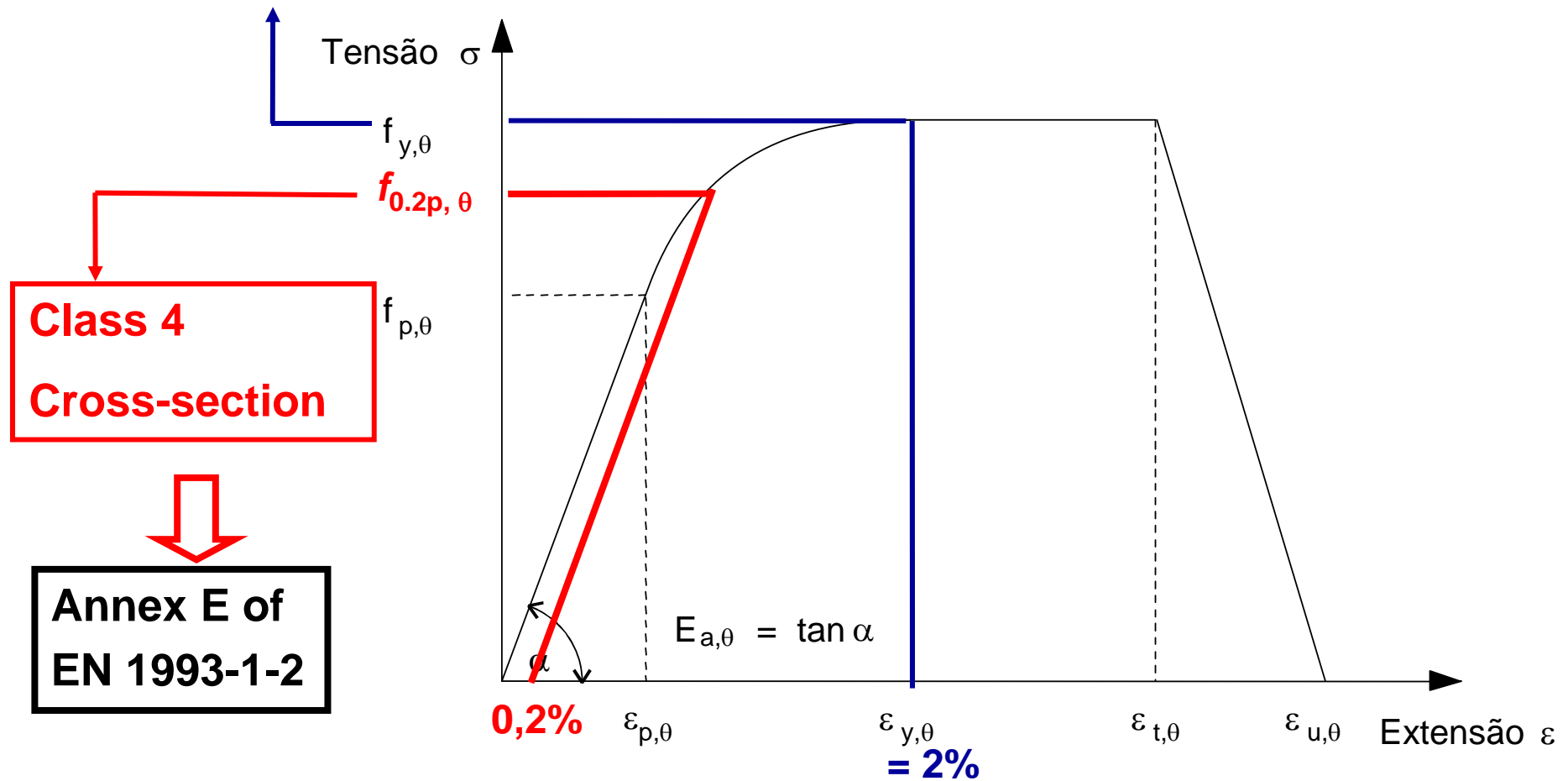
Room Temperature: $\varepsilon_{20^{\circ}C} = \sqrt{\frac{235}{f_y}}$

High Temperature:

$$\begin{aligned} \varepsilon_{\theta} &= \sqrt{\frac{235}{f_{y,\theta}}} \sqrt{\frac{E_{\theta}}{210000}} = \\ &= \sqrt{\frac{235}{k_{y,\theta} f_y}} \sqrt{\frac{k_{E,\theta} E}{210000}} = \\ &= \sqrt{\frac{k_{E,\theta}}{k_{y,\theta}}} \sqrt{\frac{235}{f_y}} \sqrt{\frac{E}{210000}} = \\ &= \sqrt{\frac{k_{E,\theta}}{k_{y,\theta}}} \sqrt{\frac{235}{f_y}} \approx 0.85 \sqrt{\frac{235}{f_y}} \end{aligned}$$



Class 1, 2 and 3 cross-section

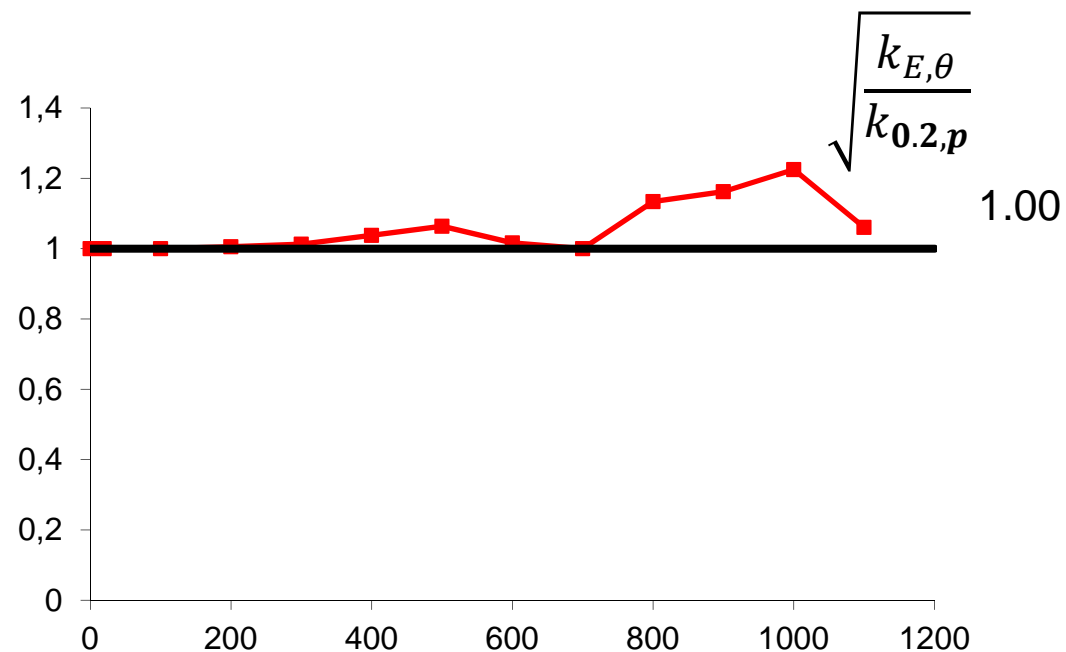




Results

High Temperature (Class 4):

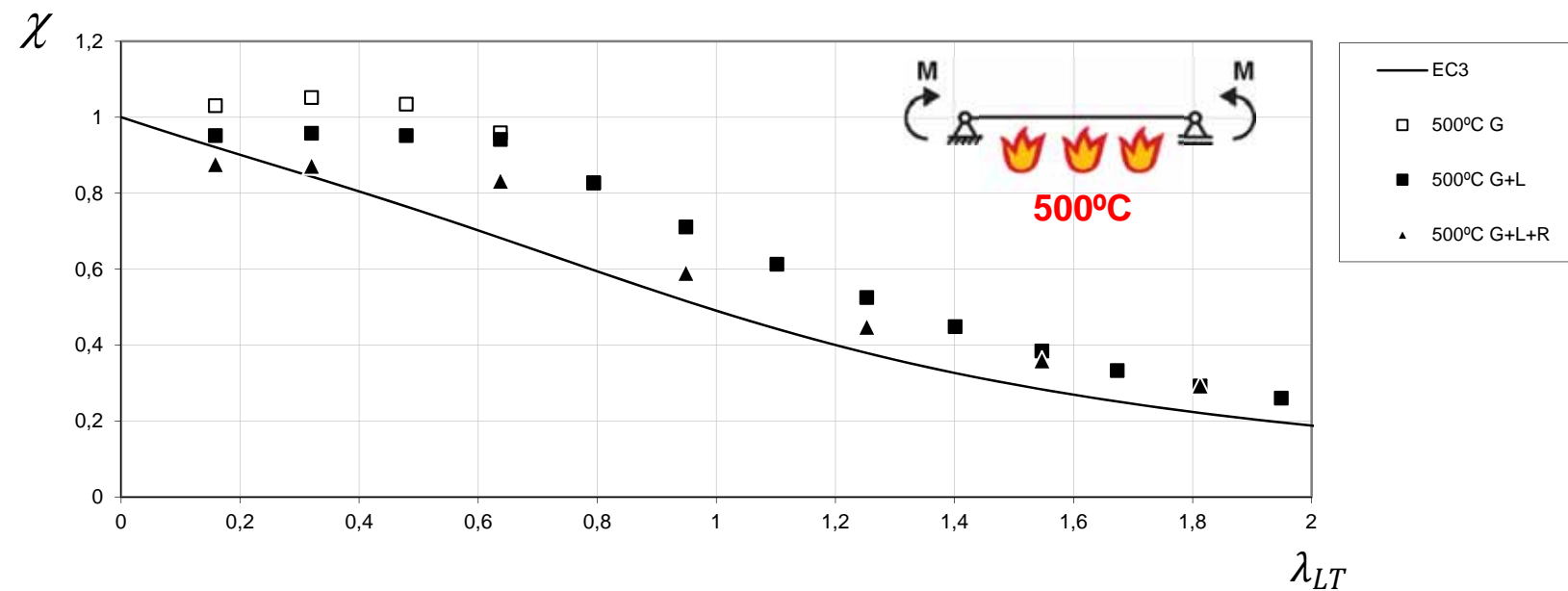
$$\varepsilon_{\theta} = \sqrt{\frac{k_{E,\theta}}{k_{0.2,p}}} \sqrt{\frac{235}{f_y}} \approx 1.00 \sqrt{\frac{235}{f_y}}$$





Results

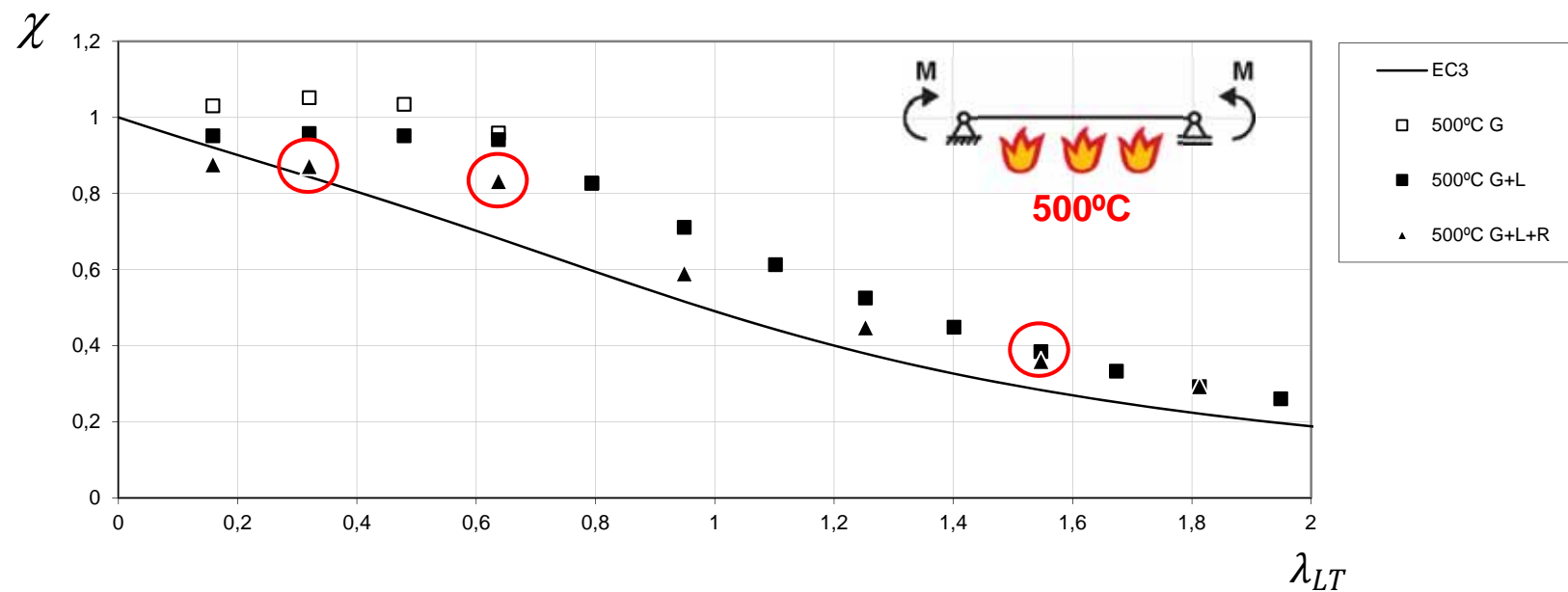
Results at 500°C with **Global Imperf. + Local imperf. + Residual Stresses**





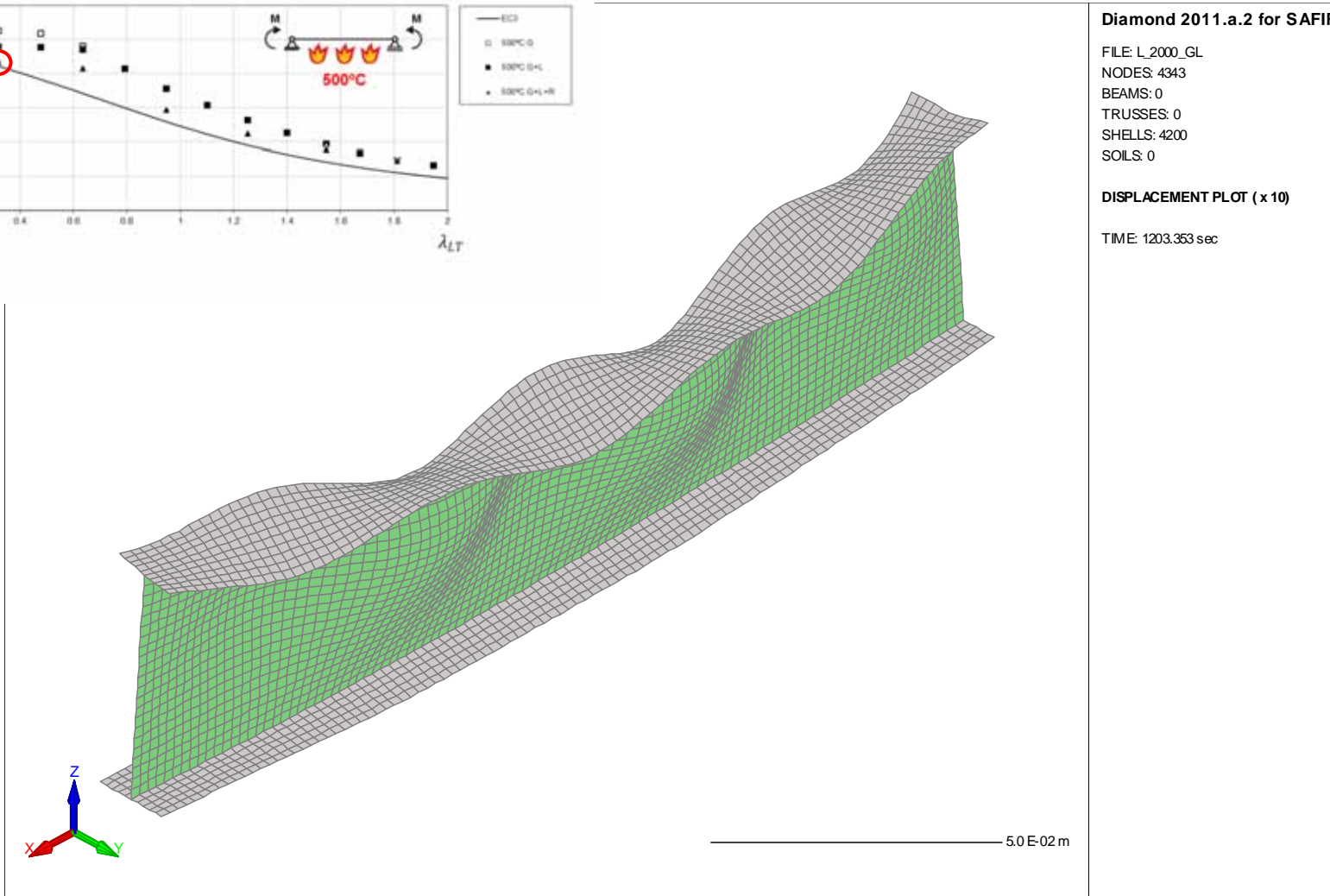
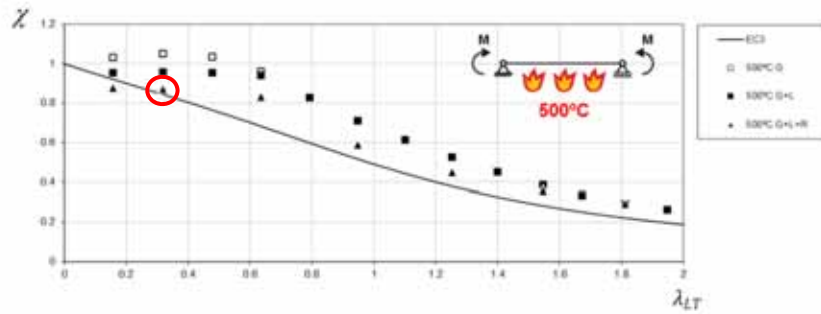
Results

Results at 500°C with **Global Imperf. + Local imperf. + Residual Stresses**



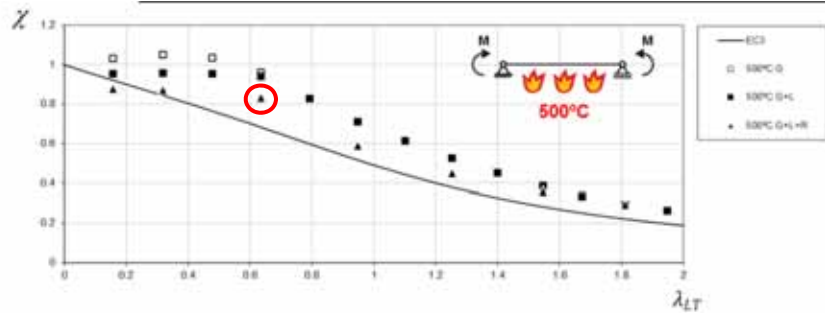


Results





Results

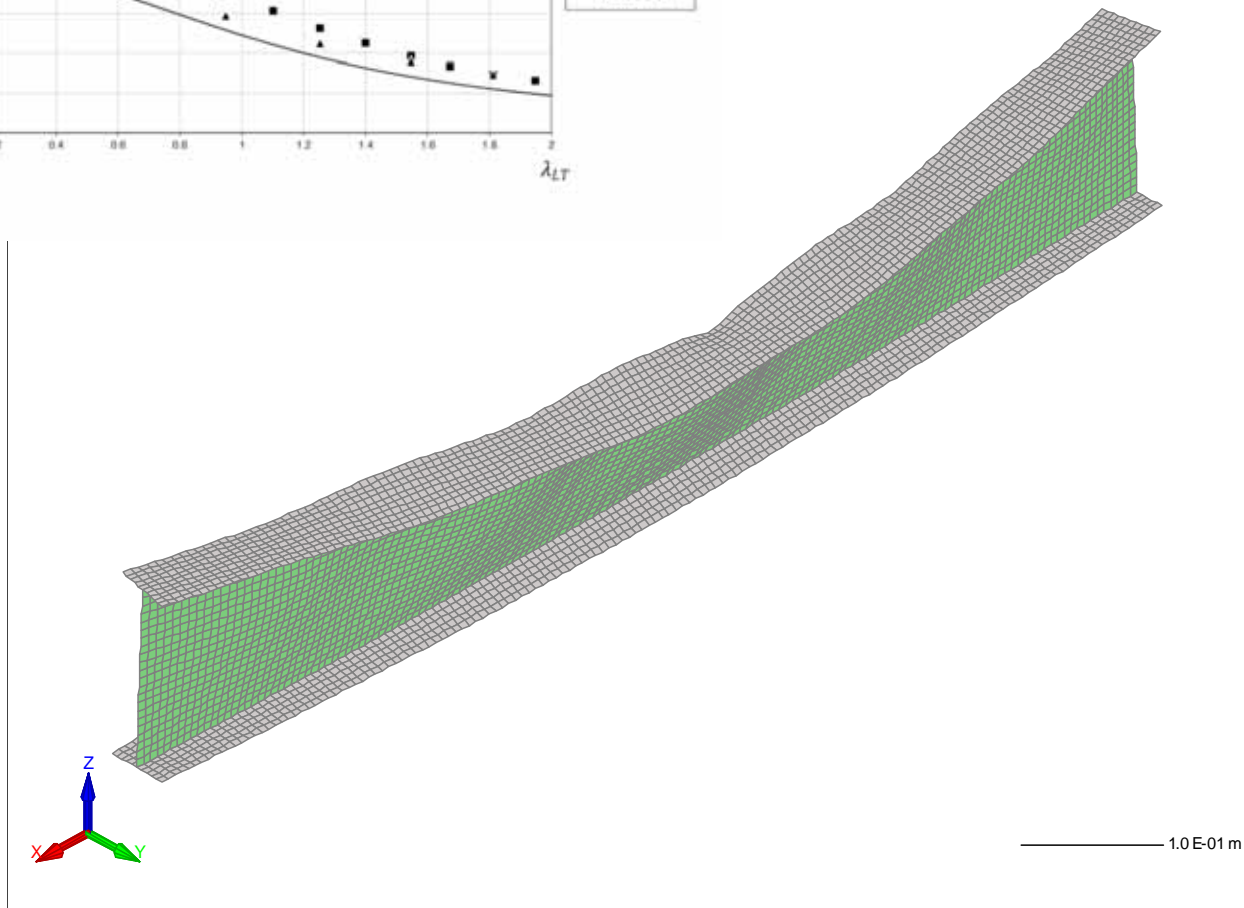


Diamond 2011.a.2 for SAFIR

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TRUSSES: 0
SHELLS: 6300
SOILS: 0

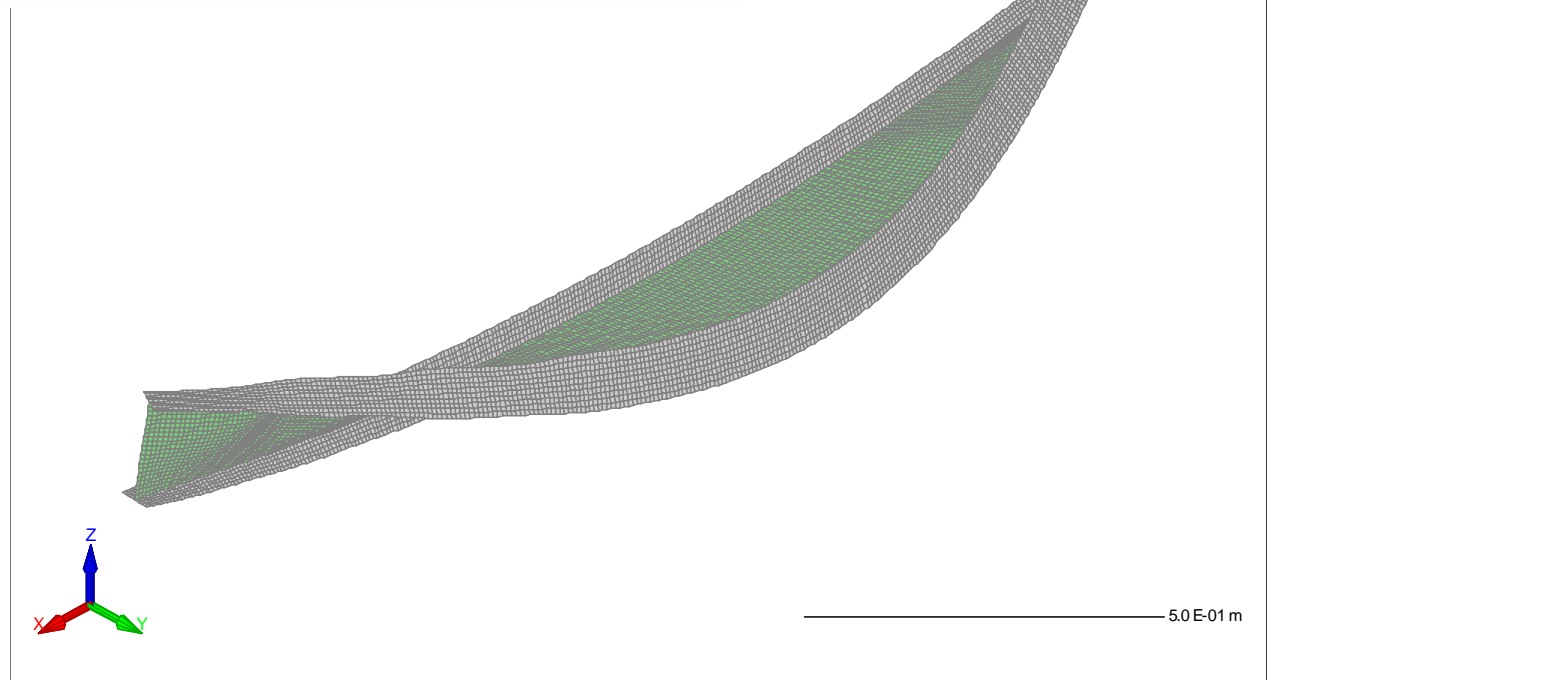
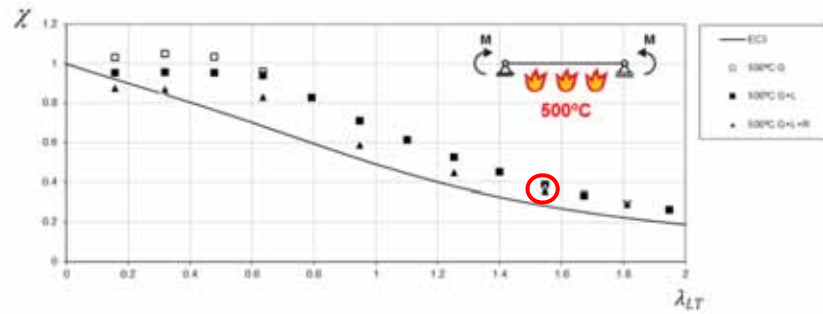
DISPLACEMENT PLOT (x 3)

TIME: 1002.427 sec





Results



COST IFR TU0904 - Training School – Malta 06-11 April 2012

THIS IS AVEIRO, PORTUGAL
See you there in 11th-12th October 2013



FIRE DESIGN OF UNIFORM AND TAPERED CLASS 4 STEEL MEMBERS

Carlos Couto

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Thank you!



decivil universidade de aveiro
departamento de engenharia civil