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Restrained Behaviour of Beams in a Steel Frame Exposed to Fire

Comparison of FEM to HCM

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Introduction

Background of the Study
The study presented here has been performed as part of a European project called COMPFIRE.

During the project fire tests were conducted on:

- Connection Components
- Isolated Joints
- Sub-frames
- Full scale buildings



Source: University of Coimbra
Source: RWTH Aachen University
Source: University of Coimbra


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Introduction

Hand calculation model

- A proposed method for hand calculation by Yin and Wang* (2005)
- 4 point concentrated loading of the beam (as in the sub-frame tests)
- The supports provide flexible rotational and axial restraints to the beam

* Yin, Y. Z., Wang, Y. C.: "Analysis of catenary action in steel beams using a simplified hand calculation method, Part 1: theory and validation for uniform temperature distribution", Journal of Constructional Steel Research, Vol. 61: pp. 183 - 211, 2005



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State of the art

Steel Structures in Fire

- Loss of Strength and Stiffness
- Thermal expansions
- Excessive deformations

Engineering approach (design codes)

- All Structural components i.e. connections, beams and columns are designed as isolated components in Fire
- For a beam the flexural resistance determines the design resistance in the absence of any interaction with surrounding structure



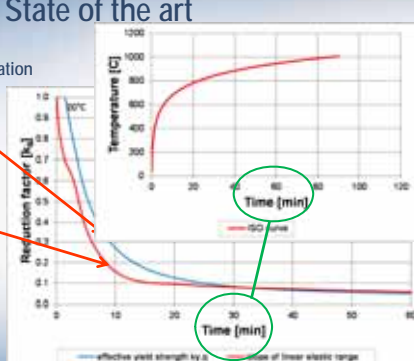

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State of the art

Material properties degradation

Yield Strength
 $f_{y,\theta} = k_{y,\theta} f_y$

Modulus of Elasticity
 $E_\theta = k_{E,\theta} E$

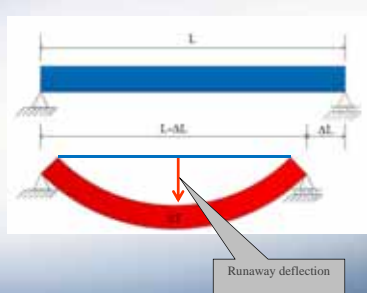




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State of the art

Conventional design

- Failure Criterion
 $M_{f,d} \leq M_{Ed,f}$
- Moment resistance
 $M_{f,d} = k_{y,\theta} \frac{W_{pl,y} f_y}{\gamma_{M,f}}$

State of the art

Restrained Beam

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State of the art

Equilibrium at elevated temperature

- Isolated beam

$$M_{Ed,fi} + M_{fi,Rd,t} = 0$$
- Restrained Beam

$$M_{Ed,fi} + M_{fi,Rd,t} + F_{axial} \times \delta = 0$$

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Benchmarking of FE-Models

FE-Model of Sub-frame

Source: University of Coimbra

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Benchmarking of FE-Models

Material model

From EN 1993 part 1-2

- Yield strength

$$f_{y,\theta} = k_{y,\theta} f_y$$
- Modulus of Elasticity

$$E_\theta = k_{E,\theta} E$$

Reduction factors provided in EN 1993-1-2

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Benchmarking of FE-Models

Simulation Steps

- Pretensioning of the bolts
 - Displacement adjustment
 - To initialize contact
- Loading of the beam
 - Pressure load
 - Area same as loading plate
- Application of heat
 - Predefined field
 - Magnitude according to measurements
 - Time vs. Temperature relation

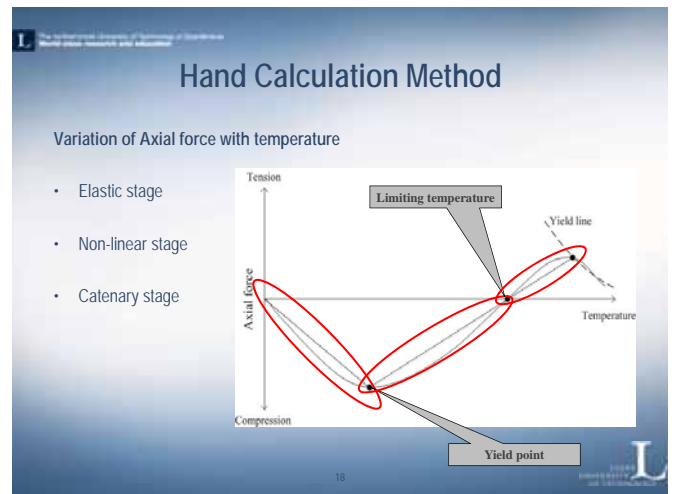
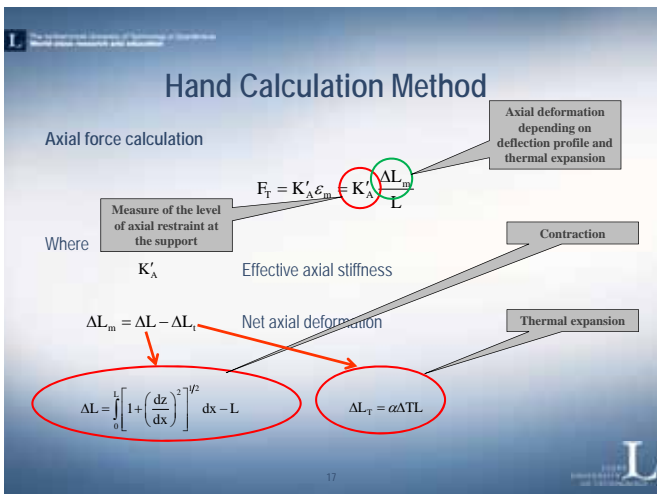
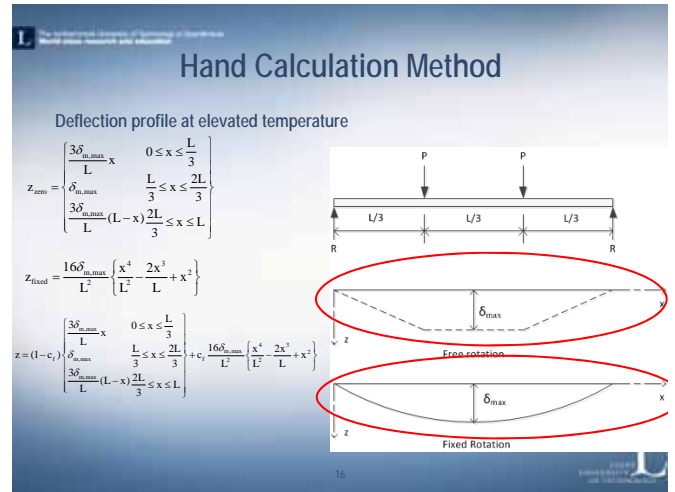
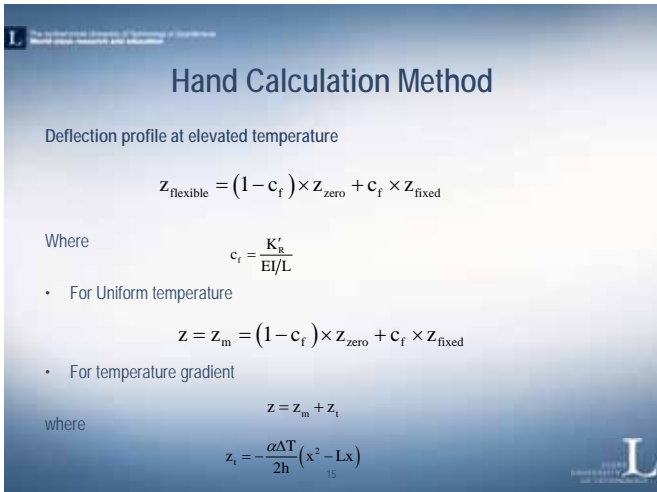
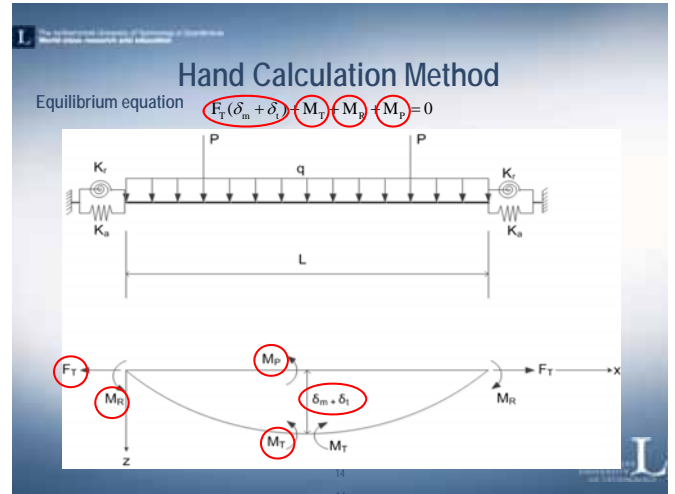
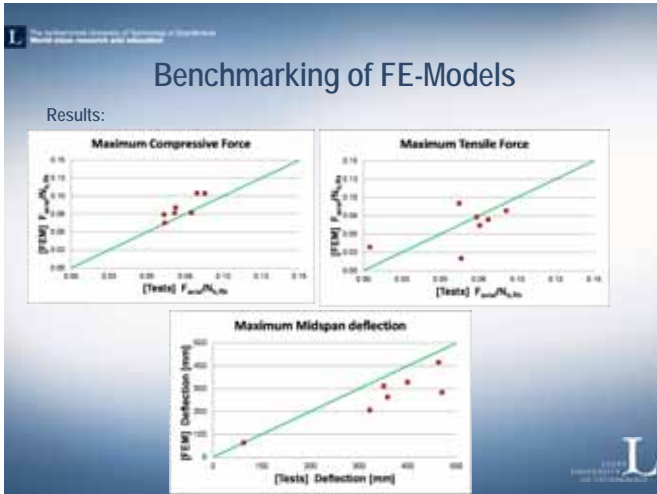
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Benchmarking of FE-Models

Artificial damping

- Material softening often causes convergence problems
- Artificial damping through 'Dissipated energy fraction'
- An optimum value for the 'Dissipated energy fraction' is required to avoid over damping
- Ratio between 'artificial strain energy' and 'total strain energy' is kept below 5 %
- The results are therefore reliable since there is no artificial increase in the total strain energy of the system
- The alternative is to use Explicite analysis in ABAQUS

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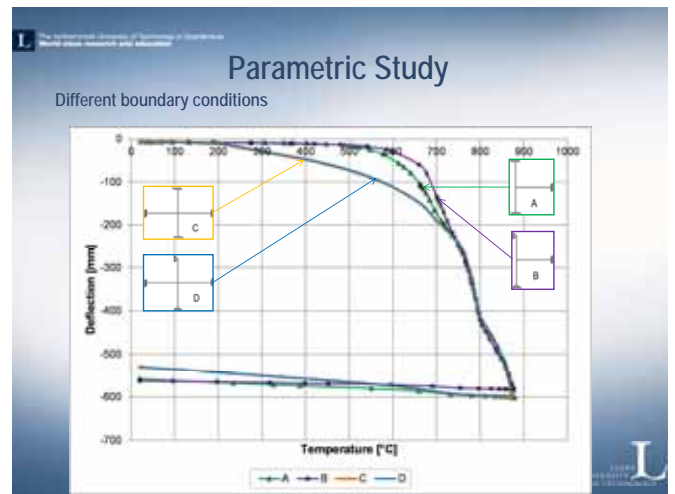
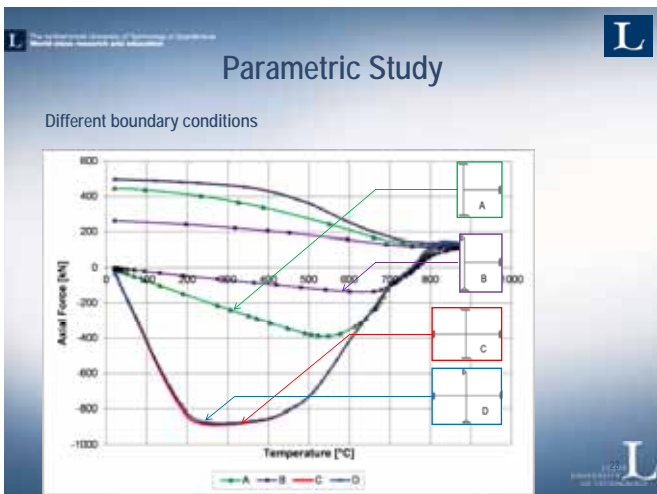
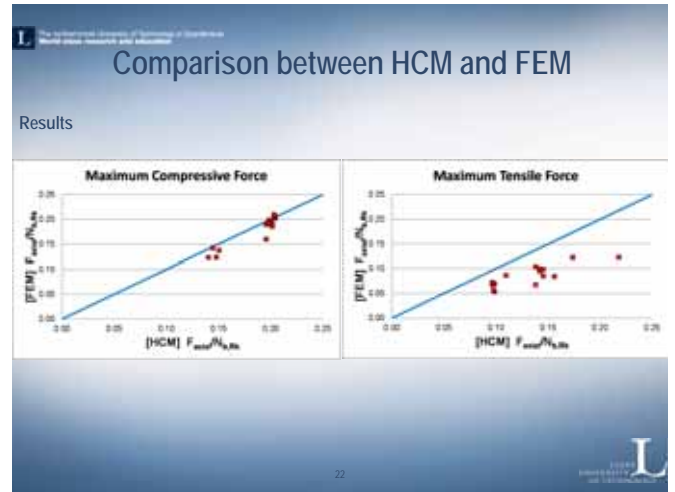
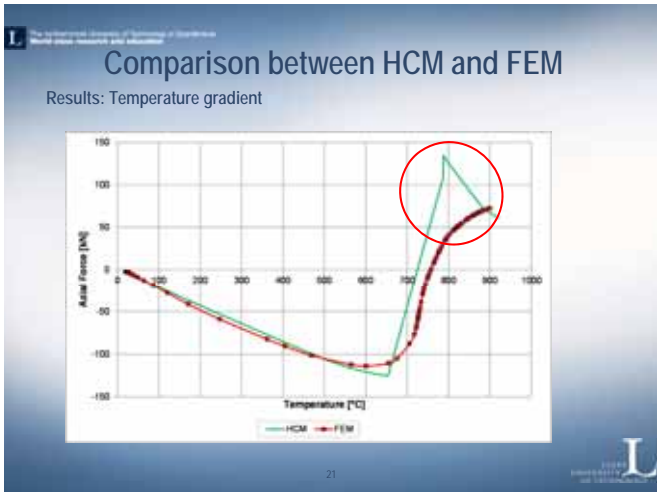
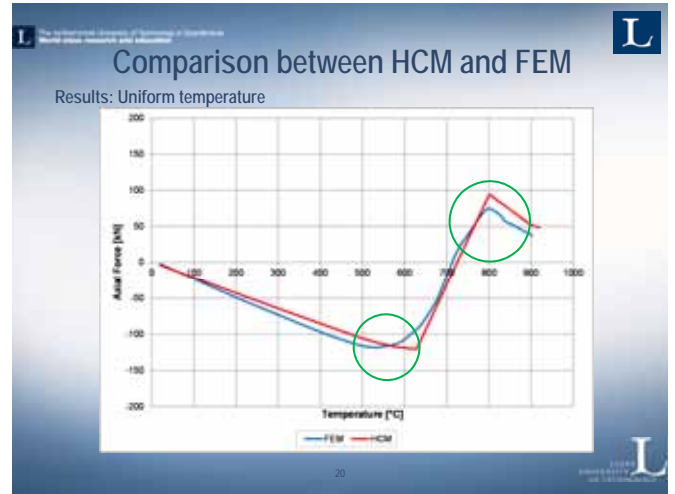


Comparison between HCM and FEM

Test Models

Sub-frame setup	Column	Beam	Connection	Beam span [m]
Setup 1	SHS 250x8	UB 178x102 x19	UK SHS 180x42.7	2
Setup 2	SHS 250x10	IPE300	U200x90x10	5
Setup 3	SHS 250x10	IPE300	U200x90x8	5
Setup 4	SHS 250x10	IPE300	U200x90x12	5

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Conclusions

- Catenary action in restrained beam provides additional resistance approx. 100°C beyond the conventional limiting temperature.
- Very high midspan deflections approx. 500 mm could be observed in the restrained beam but still be below the limit state.
- The FE-models accurately depict the axial stiffness and the maximum compression force measured in the tests, about 10% maximum difference.
- Accuracy of the maximum tensile force is slightly lower in the FE- models due to interaction with bending moment, about 25% maximum difference.
- Smaller midspan deflection in FE-Models due to slightly stiffer connections and stiffer behaviour of the FE-model in general.

